



# **Bus Connects Route 6 Lucan to City Centre – Ground Investigation**

Client: National Transport Authority (NTA)

Client's Representative: AECOM/Mott MacDonald

Report No.: 20-0399B

Date: December 2020

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### **Document Control Sheet**

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Client's Repres	entative:	AECOM/Mott MacDonald											
Revision:	A01	Status:	Final for Issue	Issue Date:	16 <sup>th</sup> December 2020								
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The works were conducted in accordance with:

British Standards Institute (2015) BS 5930:2015, Code of practice for site investigations.

BS EN 1997-2: 2007: Eurocode 7 - Geotechnical design - Part 2 Ground investigation and testing.

Geotechnical Society of Ireland (2016), Specification & Related Documents for Ground Investigation in Ireland

Laboratory testing was conducted in accordance with:

British Standards Institute BS 1377:1990 parts 2, 4, 5, 7 and 9





### METHODS OF DESCRIBING SOILS AND ROCKS

Soil and rock descriptions are based on the guidance in BS5930:2015, The Code of Practice for Site Investigation.

Abbreviations use	ed on exploratory hole logs
U	Nominal 100mm diameter undisturbed open tube sample (thick walled sampler).
UT	Nominal 100mm diameter undisturbed open tube sample (thin walled sampler).
P	Nominal 100mm diameter undisturbed piston sample.
В	Bulk disturbed sample.
LB	Large bulk disturbed sample.
D	Small disturbed sample.
С	Core sub-sample (displayed in the Field Records column on the logs).
L	Liner sample from dynamic sampled borehole.
W	Water sample.
ES / EW	Soil sample for environmental testing / Water sample for environmental testing.
SPT (s)	Standard penetration test using a split spoon sampler (small disturbed sample obtained).
SPT (c)	Standard penetration test using 60 degree solid cone.
(x,x/x,x,x,x)	Blows per increment during the standard penetration test. The initial two values relate to the seating drive (150mm) and the remaining four to the 75mm increments of the test length.
(Y for Z/Y for Z)	Incomplete standard penetration test where the full test length was not achieved. The blows 'X' represent the total blows for the given seating or test length 'Z' (mm).
N=X	SPT blow count 'N' given by the summation of the blows 'X' required to drive the full test length (300mm).
HVP / HVR	In situ hand vane test result (HVP) and vane test residual result (HVR). Results presented in kPa.
V VR	Shear vane test (borehole). Shear strength stated in kPa. V: undisturbed vane shear strength VR: remoulded vane shear strength
Soil consistency description	In cohesive soils, where samples are disturbed and there are no suitable laboratory tests, N values may be used to indicate consistency on borehole logs – a median relationship of Nx5=Cu is used (as set out in Stroud & Butler 1975).
dd-mm-yyyy	Date at the end and start of shifts, shown at the relevant borehole depth. Corresponding casing and water depths shown in the adjacent columns.
$\overline{}$	Water strike: initial depth of strike.
<b>T</b>	Water strike: depth water rose to.
Abbreviations relatin	g to rock core – reference Clause 36.4.4 of BS 5930: 2015
TCR (%)	Total Core Recovery: Ratio of rock/soil core recovered (both solid and non-intact) to the total length of core run.
SCR (%)	Solid Core Recovery: Ratio of solid core to the total length of core run. Solid core has a full diameter, uninterrupted by natural discontinuities, but not necessarily a full circumference and is measured along the core axis between natural fractures.
RQD (%)	Rock Quality Designation: Ratio of total length of solid core pieces greater than 100mm to the total length of core run.
FI	Fracture Index: Number of natural discontinuities per metre over an indicated length of core of similar intensity of fracturing.
NI	Non Intact: Used where the rock material was recovered fragmented, for example as fine to coarse gravel size particles.
AZCL	Assessed zone of core loss: The estimated depth range where core was not recovered.
DIF	Drilling induced fracture: A fracture of non-geological origin brought about by the rock coring.
(xxx/xxx/xxx)	Spacing between discontinuities (minimum/average/maximum) measured in millimetres.





## **Bus Connects Route 6 Lucan to City Centre**

#### 1 **AUTHORITY**

On the instructions of AECOM/Mott MacDonald, ("the Client's Representative"), acting on the behalf of National Transport Authority (NTA) ("the Client"), a ground investigation was undertaken at the above location to provide geotechnical and environmental information to inform the planning stage design and enable the design of Bus Connects Core Bus Corridors.

This report details the work carried out both on site and in the geotechnical and chemical testing laboratories; it contains a description of the site and the works undertaken, the exploratory hole logs and the laboratory test results.

All information given in this report is based upon the ground conditions encountered during the site investigation works, and on the results of the laboratory and field tests performed. However, there may be conditions at the site that have not been taken into account, such as unpredictable soil strata, contaminant concentrations, and water conditions between or below exploratory holes. It should be noted that groundwater levels usually vary due to seasonal and/or other effects and may at times differ to those recorded during the investigation. No responsibility can be taken for conditions not encountered through the scope of work commissioned, for example between exploratory hole points, or beneath the termination depths achieved.

This report was prepared by Causeway Geotech Ltd for the use of the Client and the Client's Representative in response to a particular set of instructions. Any other parties using the information contained in this report do so at their own risk and any duty of care to those parties is excluded.

#### 2 SCOPE

The extent of the investigation, as instructed by the Client's Representative, included boreholes, slit trenches, soil and rock core sampling, environmental sampling, groundwater monitoring, in-situ and laboratory testing, and the preparation of a factual report on the findings.

#### 3 DESCRIPTION OF SITE

As shown on the site location plan in Appendix A, the works were conducted on and alongside the N4 road from Junction 2 in the west to Ballyfermot Training Centre adjacent to the R148 (Chapilizod Bypass) in the east. The main land use in the area is residential and light commercial with Liffey Valley Shopping Centre located in the centre of the site. Large residential developments are found east through west.



#### SITE OPERATIONS

#### Summary of site works 4.1

Site operations, which were conducted between 24th September and 24th October 2020, comprised:

- six light cable percussion boreholes
- one borehole by combined percussion boring and rotary follow-on drilling
- six boreholes by dynamic (windowless) sampling methods
- a standpipe installation in one borehole
- one slit trench

The exploratory holes and in-situ tests were located as instructed by the Client's Representative, as shown on the exploratory hole location plan in Appendix A.

#### 4.2 **Boreholes**

A total of thirteen boreholes were put down in a minimum diameter of 150mm through soils and rock strata to their completion depths by a combination of methods, including light percussion boring using Dando Terrier rigs, light cable percussion boring by Dando 2000 rigs, and rotary drilling (by Hanjin D8 rotary drilling rigs).

The borehole logs state the methodology and plant used for each location, as well as the appropriate depth ranges.

A summary of the boreholes, subdivided by category in accordance with the methods employed for their completion, is presented in the following sub-sections.

#### 4.2.1 Light cable percussion boreholes

Six boreholes (R6-CP01, R6-CP03 and R6-CP08 - R6-CP11) were put down to completion in minimum 200mm diameter using Dando 2000 light cable percussion boring rigs. All boreholes were terminated on encountering virtual refusal on obstructions.

Hand dug inspection pits were carried out between ground level and 1.20m depth to ensure boreholes were put down at locations clear of services or subsurface obstructions.

Disturbed (bulk and small bag) samples were taken within the encountered strata. Undisturbed (U100) samples were taken where appropriate and as directed within fine soils. Environmental samples were





taken at standard intervals, as directed by the Client's Representative.

Standard penetration tests were carried out in accordance with BS EN 22476-3:2005+A1:2011 at standard depth intervals using the split spoon sampler ( $SPT_{(s)}$ ) or solid cone attachment ( $SPT_{(c)}$ ). The penetrations are stated for those tests for which the full 150mm seating drive or 300mm test drive was not possible. The N-values provided on the borehole logs are uncorrected and no allowance has been made for energy ratio corrections. The SPT hammer energy measurement report is provided in Appendix H.

Any water strikes encountered during boring were recorded along with any changes in their levels as the borehole proceeded.

Appendix B presents the borehole logs.

#### 4.2.2 Boreholes by combined percussion boring and rotary follow-on drilling

One borehole (R6-CP07) was put down by a combination of light percussion boring and rotary follow-on drilling techniques with core recovery in bedrock. Where the light percussion borehole had not been advanced onto bedrock, rotary percussive methods were employed to advance the borehole to completion/bedrock. Symmetrix cased full-hole drilling was used, with SPTs carried out at standard intervals as required.

Hand dug inspection pits were carried out between ground level and 1.20m depth to ensure boreholes were put down at locations clear of services or subsurface obstructions.

Standard penetration tests were carried out in accordance with BS EN 22476-3:2005+A1:2011 at standard depth intervals throughout the overburden using the split spoon sampler ( $SPT_{(s)}$ ) or solid cone attachment ( $SPT_{(c)}$ ). The penetrations are stated for those tests for which the full 150mm seating drive or 300mm test drive was not possible. The N-values provided on the borehole logs are uncorrected and no allowance has been made for energy ratio corrections. The SPT hammer energy measurement report is provided in Appendix H.

Where coring was carried out within bedrock strata, Geobor S Coring was used. The core was extracted in up to 1.5m lengths using a SK6L core barrel, which produced core of nominal 102mm diameter, and was placed in single channel wooden core boxes.

The core was subsequently photographed and examined by a qualified and experienced Engineering Geologist, thus enabling the production of an engineering log in accordance with *BS 5930: 2015: Code of practice for ground investigations*.

Appendix B presents the borehole logs, with core photographs presented in Appendix C.





#### 4.2.3 Dynamic sampled boreholes

Six boreholes (R6-CP02, R6-CP04 – R6-CP06, R6-WS01 & R6-WS02) were put down to completion by light percussion boring techniques using a Dando Terrier dynamic sampling rig. The boreholes were put down initially in 150mm diameter, reducing in diameter with depth as required, down to 50mm by use of the smallest sampler.

Hand dug inspection pits were carried out between ground level and 1.20m depth to ensure boreholes were put down clear of services or subsurface obstructions. The boreholes were taken to depths ranging between 0.87m and 3.60m where they were terminated on encountering virtual refusal on obstructions.

Standard penetration tests were carried out in accordance with BS EN 22476-3:2005+A1:2011 at standard depth intervals using the split spoon sampler ( $SPT_{(s)}$ ) or solid cone attachment ( $SPT_{(c)}$ ). The penetrations are stated for those tests for which the full 150mm seating drive or 300mm test drive was not possible. The N-values provided on the borehole logs are uncorrected and no allowance has been made for energy ratio corrections. The SPT hammer energy measurement report is provided in Appendix H.

Disturbed (bulk and small bag) samples were taken within the encountered strata. Environmental samples were taken at standard intervals, as directed by the Client's Representative. Undisturbed (U100) samples were taken as appropriate within fine grained strata.

Any water strikes encountered during boring were recorded along with any changes in their levels as the borehole proceeded. Details of the water strikes are presented on the individual borehole logs.

Appendix B presents the borehole logs.

#### 4.3 Dynamic probes

Two dynamic probes (R6-CP02DP and R6-CP07DP) were conducted as a follow on from the boreholes using the DPSHB method as described in BS EN ISO 22476-3:2005+A1:2011. The method entails a 63.5kg hammer falling 0.75m onto a 50.5mm diameter cone with an apex angle of 90°.

Appendix B provides the dynamic probe logs on the sheet following the relevant borehole log in the form of plots, against depth, of the number of blows per 100mm penetration.

#### 4.4 Standpipe installations

A groundwater monitoring standpipe was installed in boreholes R6-CP07

Details of the installations, including the depth range of the response zone, are provided in Appendix B on the individual borehole logs.



#### 4.5 Slit trenches

One slit trench (R6-TP01) was excavated by a combination of hand digging and mechanical excavation using a compact 3t tracked excavator fitted with a 600mm wide toothless bucket, to locate and identify buried services at the site. An attempt was also made to investigate foundations of existing bridge abutments at this location. A concrete projection was identified at 1.10mbgl however it was not possible to establish or confirm foundation makeup.

Drawing of the trenches and the locations of services encountered during excavation are shown along with the slit trench logs in Appendix D, with photographs presented in Appendix E.

#### 4.6 Surveying

The as-built exploratory hole positions were surveyed following completion of site operations by a Site Engineer from Causeway Geotech. Surveying was carried out using a Trimble R6 GPS system employing VRS and real time kinetic (RTK) techniques.

The plan coordinates (Irish Transverse Mercator) and ground elevation (mOD Malin) at each location are recorded on the individual exploratory hole logs. The exploratory hole plan presented in Appendix A shows these as-built positions.

#### 4.7 Groundwater monitoring

Following completion of site works, a round of groundwater monitoring was carried out. Ground water monitoring was carried out using a water interface probe.

#### 5 LABORATORY WORK

Upon their receipt in the laboratory, all disturbed samples were carefully examined and accurately described and their descriptions incorporated into the borehole logs.

#### 5.1 Geotechnical laboratory testing of soils

Laboratory testing of soils comprised:

- **soil classification:** moisture content measurement, Atterberg Limit tests and particle size distribution analysis.
- **shear strength** (total stress): unconsolidated undrained triaxial tests
- soil chemistry: pH and water soluble sulphate content

Laboratory testing of soils samples was carried out in accordance with British Standards Institute: *BS 1377, Methods of test for soils for civil engineering purposes; Part 1 (2016), and Parts 2-9 (1990).* 

The test results are presented in Appendix F.

### 5.2 Geotechnical laboratory testing of rock

Laboratory testing of rock sub-samples comprised:

- point load index
- unconfined compressive strength (UCS) tests

Test	Test carried out in accordance with										
Point load index	ISRM Suggested Methods (1985) Suggested method for determining point-load strength. Int. J. Rock Mech. Min. Sci. Geomech. Abstr. 22, pp. 53–60										
	strength. Inc. J. Rock Mech. Mili. Sci. Geomech. Abstr. 22, pp. 33-00										
Uniaxial	ISRM Suggested Methods (1981) Suggested method for determining										
compression	deformability of rock materials in uniaxial compression, Part 2										
strength tests	and										
	ISRM (2007) Ulusay R, Hudson JA (eds) The complete ISRM suggested methods										
	for rock characterization, testing and monitoring, 2007										

The test results are presented in Appendix F.

#### 5.3 Environmental laboratory testing of soils

Environmental testing, as specified by the Client's Representative was conducted on selected environmental soil samples by Chemtest at its laboratory in Newmarket, Suffolk.

Testing was carried out on a number of samples according to Engineer's Ireland Suite E and Suite H including testing for a range of determinants:

- Metals
- Speciated total petroleum hydrocarbons (TPH)
- Speciated polycyclic aromatic hydrocarbons (PAH)
- Cyanides
- Asbestos screen
- pH
- Waste acceptance criteria (WAC) testing

Results of environmental laboratory testing are presented in Appendix G.





#### **6 GROUND CONDITIONS**

#### 6.1 General geology of the area

Published geological mapping indicate the superficial deposits underlying the site comprise mainly Glacial Till. These deposits are underlain by limestone and shale of the Lucan Formation. An area of gravels derived from limestone underlain by the Waulsortian Limestones was noted on the site's western boundary. East of the M50 on the Chapelizod Bypass is underlain by made ground, alluvium and glacial till.

### 6.2 Ground types encountered during investigation of the site

A summary of the ground types encountered in the exploratory holes is listed below, in approximate stratigraphic order:

- Paved surface: boreholes R6-CP10 & R6-CP11 encountered 0.3mm of macadam surfacing. In addition, R6-TP01 had concrete down to 100mm. Concrete was encountered at 4.0mbgl in borehole R6-CP10
- **Topsoil:** the remainder of the site encountered topsoil in 0.1m-0.3m thickness.
- **Made Ground (sub-base):** approximately 1.0m of aggregate fill beneath the paved surfaces.
- **Made Ground (fill):** reworked sandy gravelly clay fill occasionally with fragments of concrete extending to a maximum depth of 4.0m at bridge abutment locations.
- **Alluvium:** very soft clay encountered at depths ranging 3.0m-4.0m in borehole R6-CP09
- **Glacial Till:** sandy gravelly clay, typically firm or stiff in upper horizons, becoming very stiff with increasing depth.
- Bedrock (Limestone): Rockhead was encountered at a depth of 7.6m in R6-CP07.

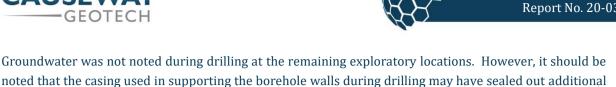
#### 6.3 Groundwater

Details of the individual groundwater strikes, along with any relative changes in levels as works proceeded, are presented on the exploratory hole logs for each location.

Groundwater was encountered during percussion boring through soil as water strikes at 2.5m – 3.1m in boreholes R6-CP03 – R6-CP05 and R6-CP07. Groundwater was encountered in slit trench R6-TP02.



works should not be ruled out.



It should also be noted that any groundwater strikes within bedrock may have been masked by the fluid used as the drilling flush medium.

groundwater strikes and the possibility of encountering groundwater at other depths during excavation

Subsequent groundwater monitoring of the standpipe installation recorded water levels as shown in Table 1.

**Table 1: Groundwater monitoring** 

Date	Water level (mbgl)
Date	R6-CP07
19/11/2020	6.35

Continued monitoring of the installed standpipe will give an indication of the seasonal variation in groundwater level which should be factored into design considerations.

#### 7 REFERENCES

Geotechnical Society of Ireland (2016), Specification & Related Documents for Ground Investigation in Ireland

IS EN 1997-2: 2007: Eurocode 7 - Geotechnical design - Part 2 Ground investigation and testing. National Standards Authority of Ireland.

BS 5930: 2015: Code of practice for ground investigations. British Standards Institution.

BS EN ISO 14688-1:2018: Geotechnical investigation and testing. Identification and classification of soil. Part 1 Identification and description.

BS EN ISO 14688-2:2018: Geotechnical investigation and testing. Identification and classification of soil. Part 2 Principles for a classification.

BS 1377: 1990: Methods of test for soils for civil engineering purposes. British Standards Institution.

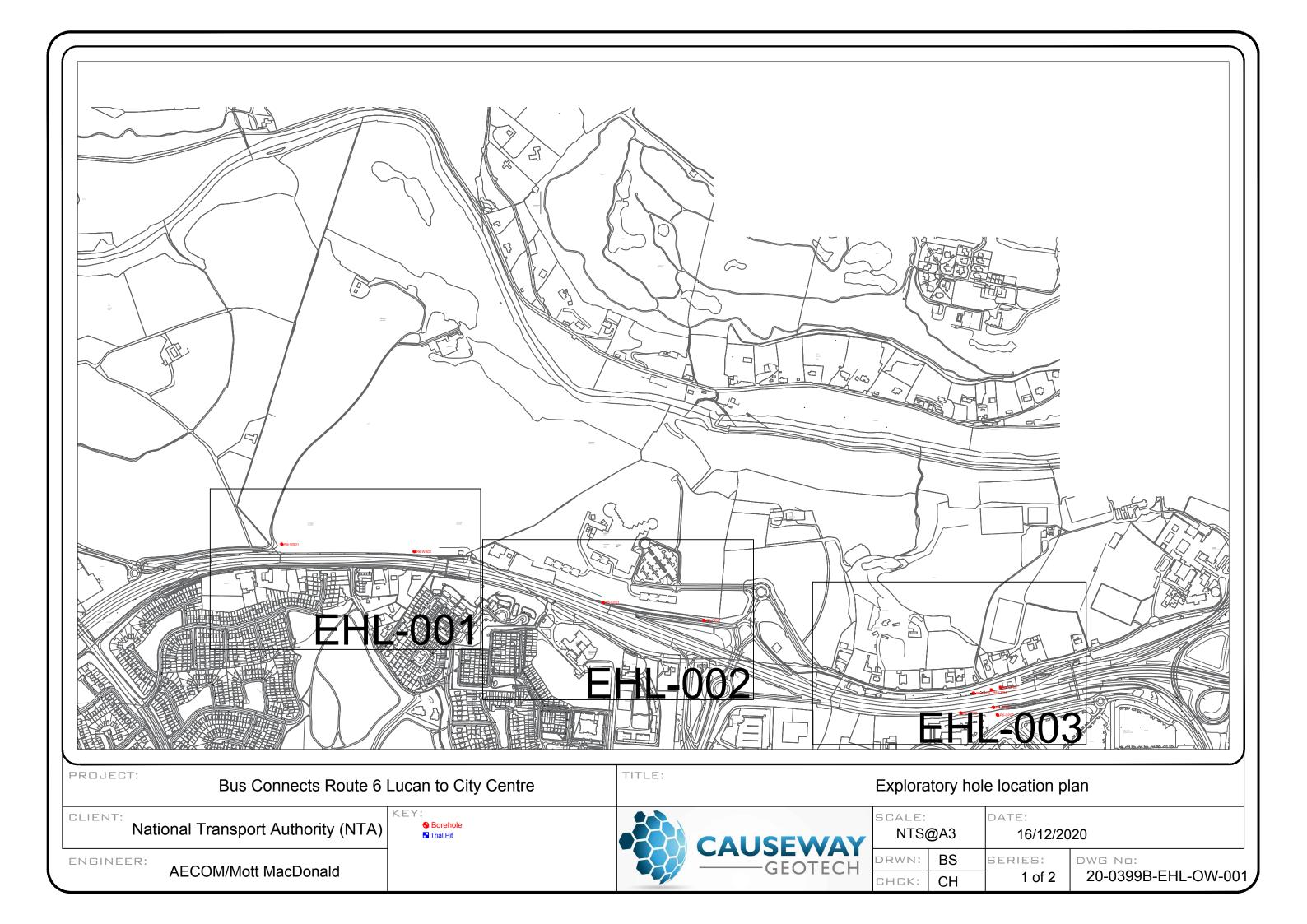
BS EN ISO 14689-1:2018: Geotechnical investigation and testing. Identification and classification of rock. Identification and description.

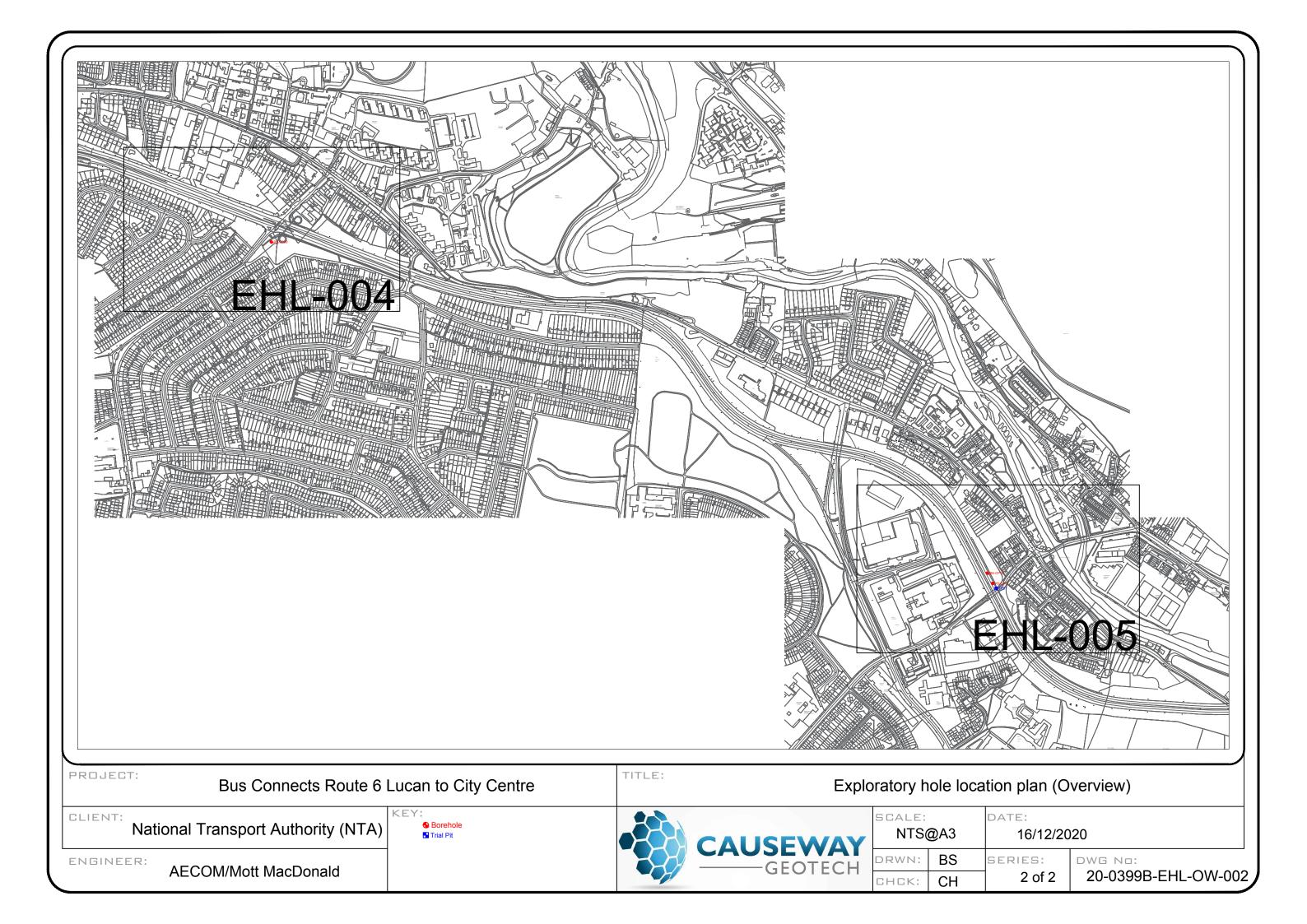
BS EN ISO 22476-3:2005+A1:2011: Geotechnical investigation and testing. Field testing. Standard penetration test.

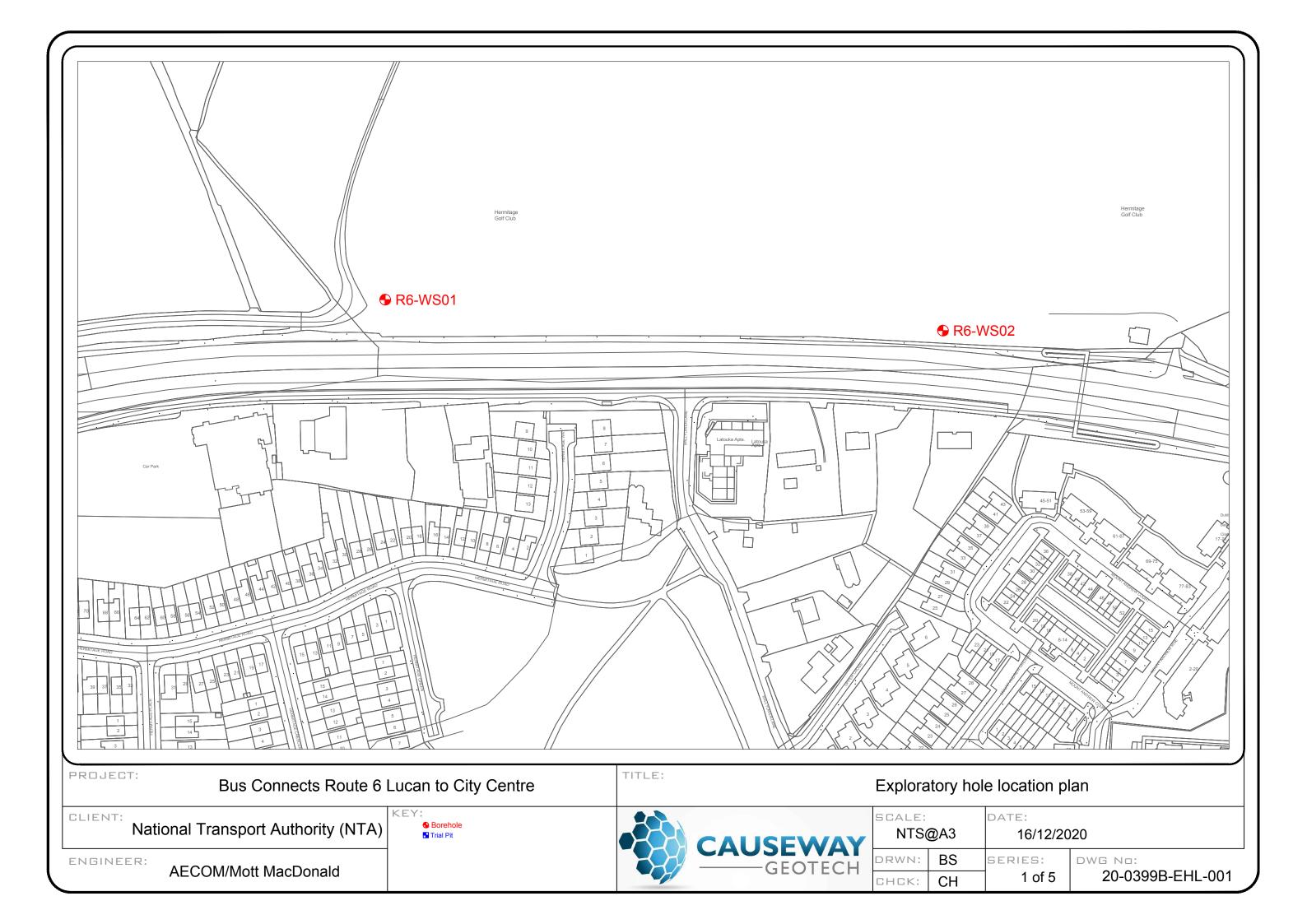


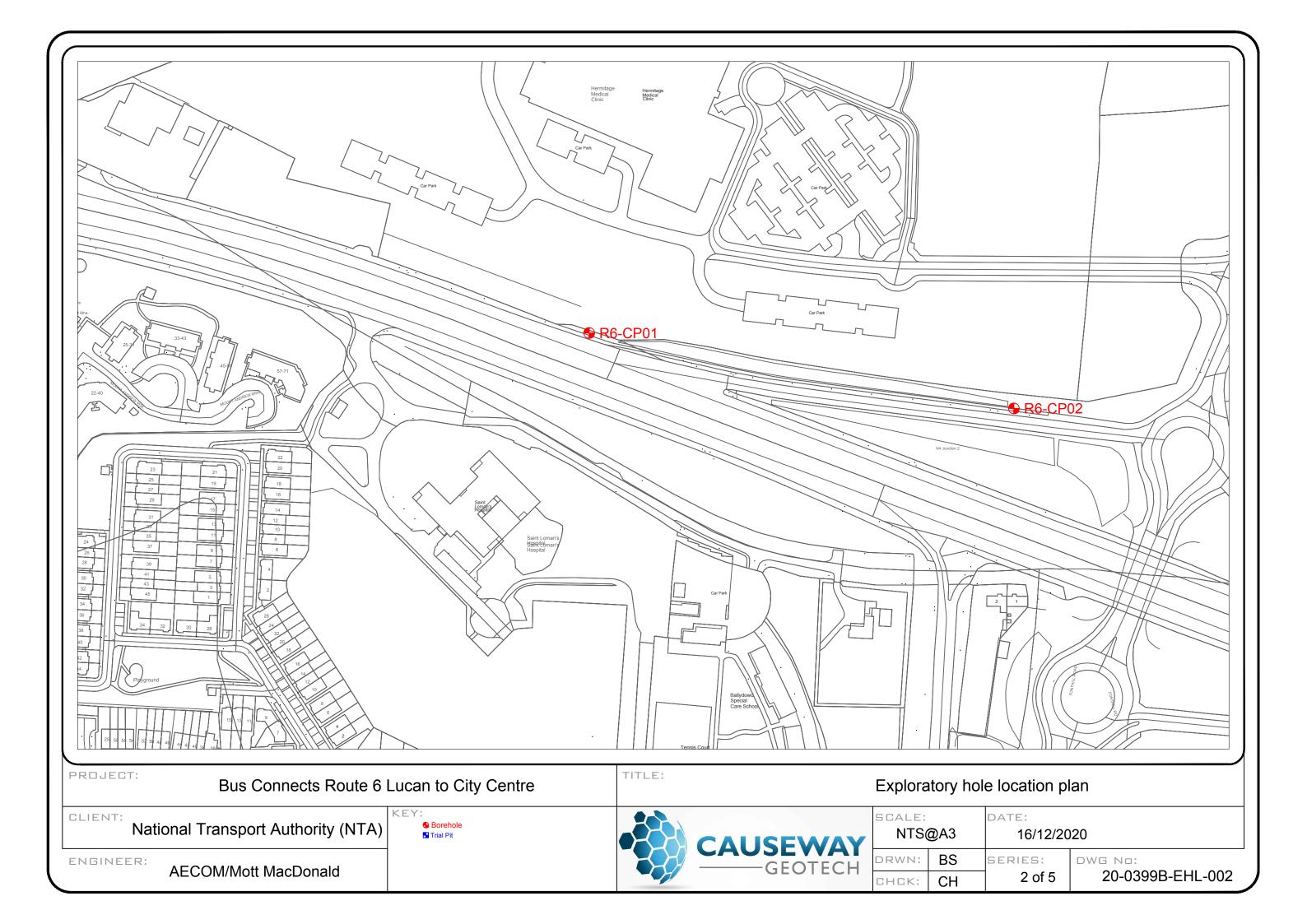
# APPENDIX A EXPLORATORY HOLE LOCATION PLAN

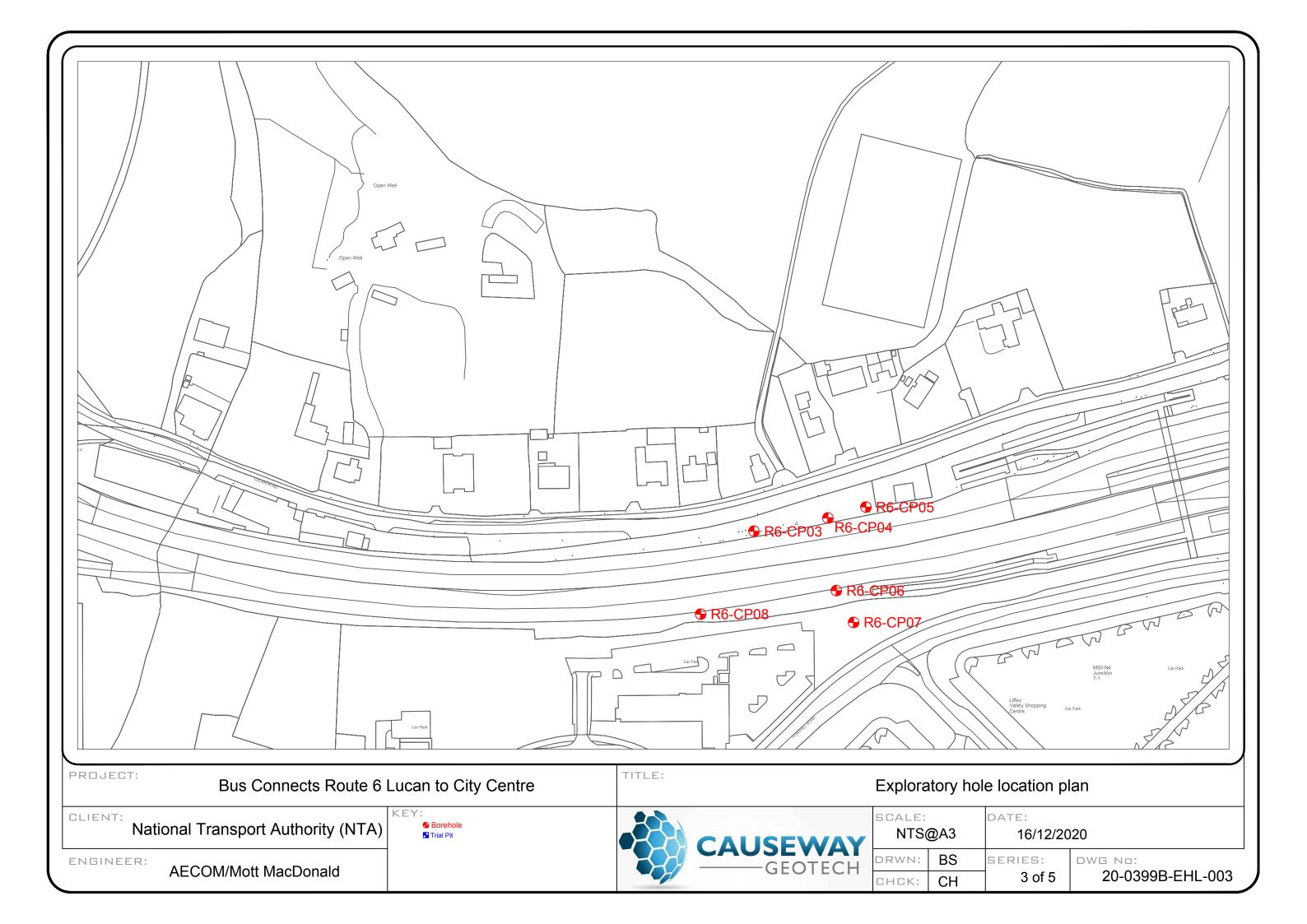


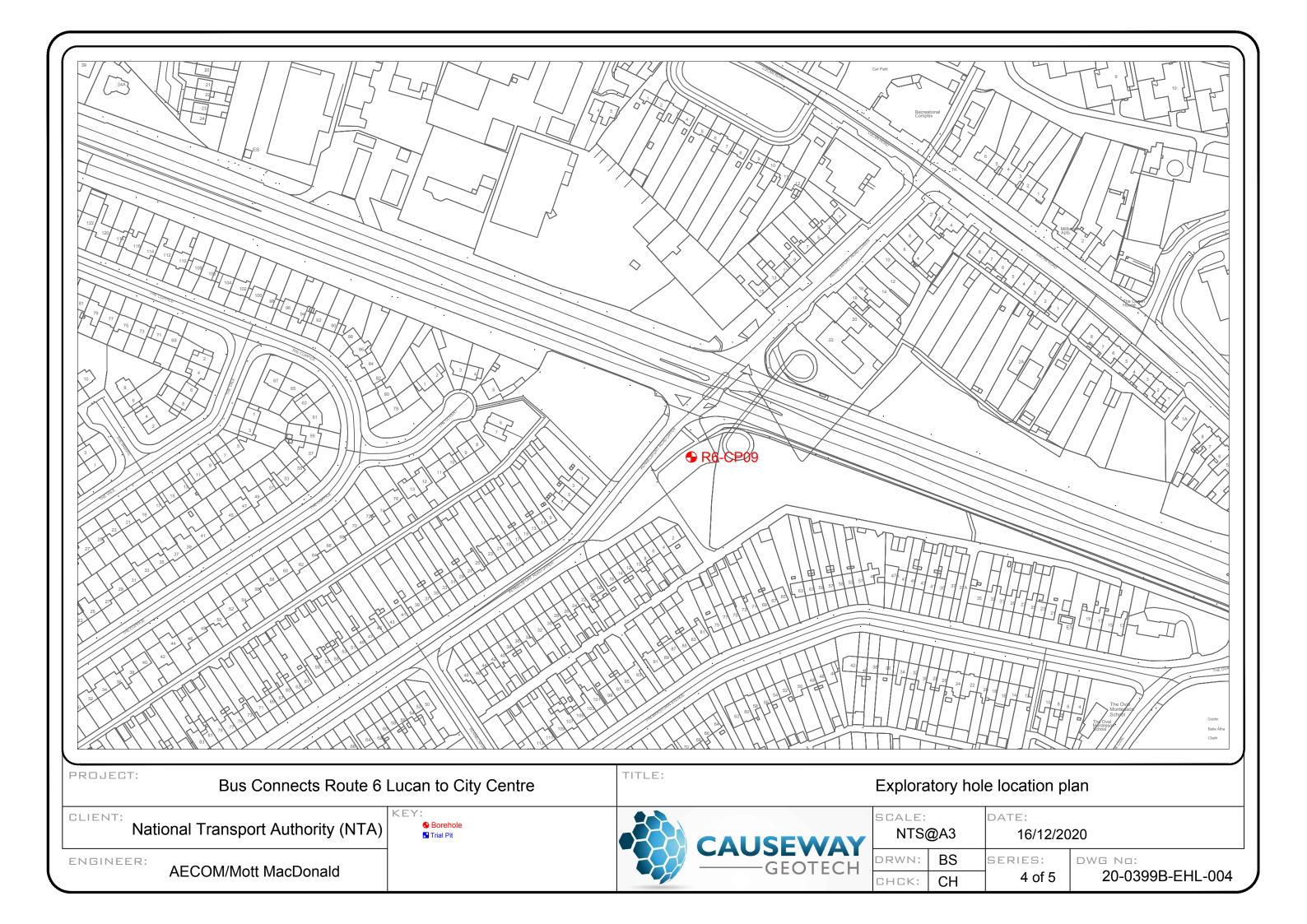


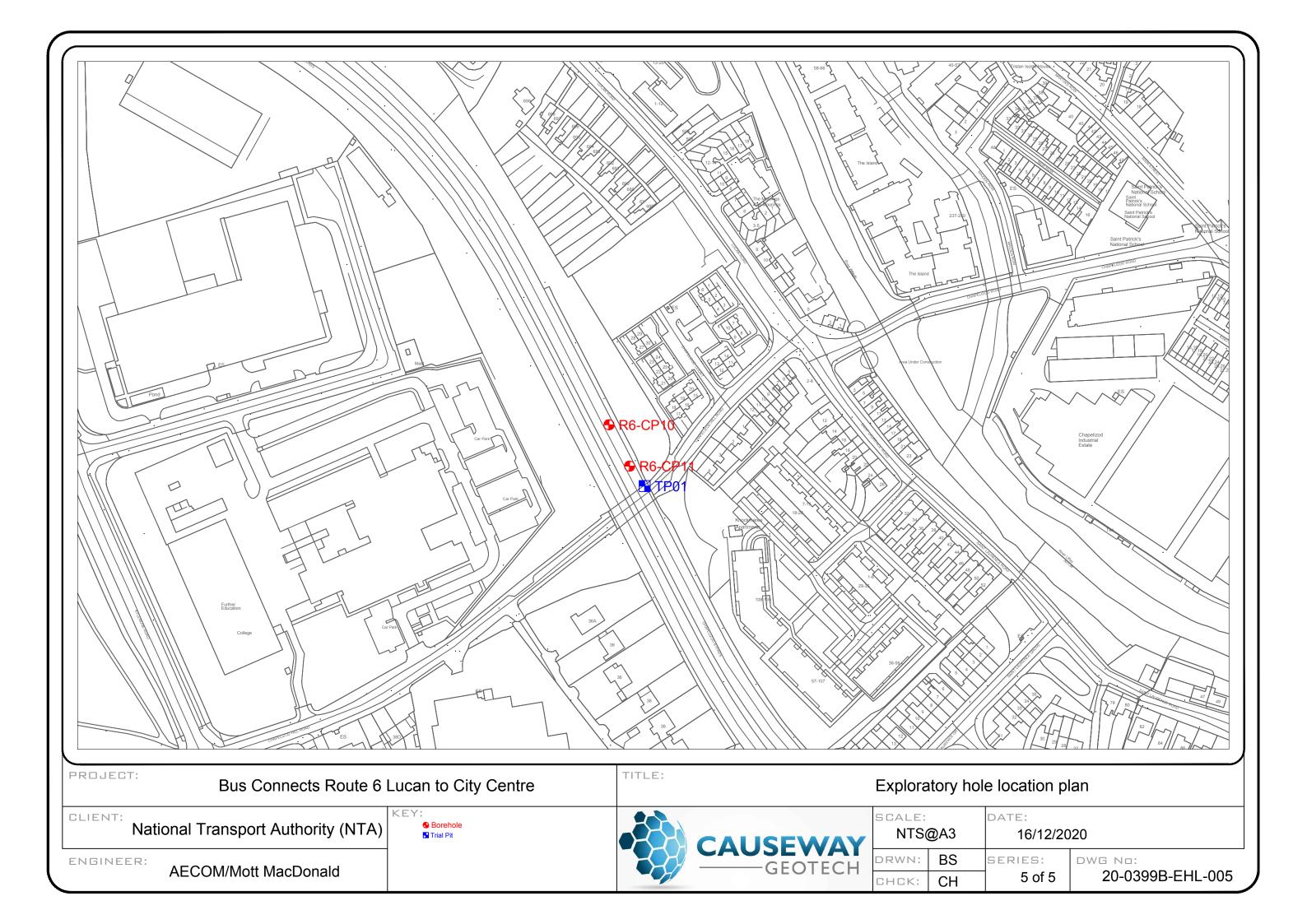














APPENDIX B
BOREHOLE LOGS



	CAUSEW	ECH				ect No. 399B	Project Client: Client's	National	nects Route 6 Lucai Transport Authorit 'Mott MacDonald			orehole R6-CP0	
Method Cable Percussion	Plant Used Dando 2000	<b>Top (m)</b> 0.00	Base 5.0	-		dinates 72.26 E	Final De	<b>epth:</b> 5.00 m	<b>Start Date:</b> 19/10/2	Driller: BM		heet 1 of Scale: 1:4	
						31.46 N	Elevatio	on: 60.62 mOD	End Date: 19/10/2	Logger: CH		FINAL	
Depth Sample (m) Tests	Field Records	i	Casing Depth (m)	Water Depth (m)	Level mOD 60.52	Depth (m)	Legend	TOPSOIL	Description		Water	Backfill	-
.50 B5 .50 ES1 .00 B6 .00 ES2 .20 D10 .20 - 1.65 SPT (S	N=8 (2,2/2,2,2,2) Ham 0643	1.20	Dry	59.62	- 1.00	X - 15 - 15 - 15 - 15 - 15 - 15 - 15 - 1	MADE GROUND: So coarse. Gravel is sul lithologies. Firm brown slightly		I fine to coarse of mixed  Y. Sand is fine to coarse.			1.0	
00 B7 00 D11 00 ES3 00 - 2.45 SPT (S	N=9 (4,2/2,2,2,3) Ham 0643	mer SN =	1.50	Dry			X - X - X - X - X - X - X - X - X - X -						2.0
.00 B8 .00 ES4 .00 - 3.45 U14	Ublow=30 80%		1.50	Dry			× × × × × × × × × × × × × × × × × × ×						3.0
.00 B9 .00 D12 .00 - 4.45 SPT (S	N=33 (5,5/7,8,8,10) Ha = 0643	ammer SN	1.50	Dry	56.62	4.00	***************************************		gravelly CLAY. Sand is fin ounded fine to coarse of				4.0
.00 D13 .00 - 5.10 SPT (S	) N=50 (25 for 50mm/50 50mm) Hammer SN = (		1.50	Dry	55.92 55.62	- 4.70 - 5.00		Grey sandy angular coarse. (Possible be	coarse GRAVEL of limes drock) End of Borehole at 5				5.0
	er Strikes m) Time (min) Rose to (r	m) From 4.7(	(m)	elling To (i		e (hh:mm)		nspection pit excavat water encountered.	ed to 1.20m.				6.5
Casing Details To (m) Diamete 1.50 200	Water Added r From (m) To (m)							i <b>on Reason</b> d on refusal.		<b>Last Updated</b> 16/12/2020		AG	_

	C	AUSEM	/AY				ct No. 399B	Project Client:		nects Route 6 Lucan to Transport Authority (N		Borehole ID R6-CP02				
		GEOT	ЕСП					Client's	Rep: AECOM/	Mott MacDonald						
Metho Light Percu		Plant Used Dando Terrier	<b>Top (m</b> 0.00	1.9			inates	Final De	<b>epth:</b> 1.90 m	<b>Start Date:</b> 22/10/2020	<b>Driller:</b> JC		Sheet 1 of 1 Scale: 1:50			
							5.22 E 0.10 N	Elevatio	on: 52.79 mOD	End Date: 22/10/2020	Logger: CH		FINAL			
Depth (m)	Sample / Tests	Field Records	5	Depth D	ater epth (m)	Level mOD	Depth (m)	Legend	1	Description	,	Water	Backfill			
20 - 0.70	B1					52.59	0.20			MADE GROUND: Grey angular to subangular fine to coarse GRAVEL of mixed lithologies. Sand is fine to coarse.						
70 - 1.20	B2					52.09	0.70			ravelly CLAY. Sand is fine to counded fine to coarse of mix						
0	ES3						-		Subungular to Subre	variated fille to course of fills	ed intilologies.		1.			
20 - 1.90 20 - 1.65		N=29 (5,7/6,7,8,8) Hai 0696	mmer SN :	0.00		51.59	1.20	× × ×		gravelly sandy silty CLAY. San r to subrounded fine to med			1.			
90 90 - 2.35		N=38 (6,9/9,11,11,7) F SN = 0696	Hammer	0.00		50.89	- - 1.90 -	X		End of Borehole at 1.90m	l		2.			
							- - - -						2.			
							-						3.			
							- - - -						3.			
							-						4.			
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							_ _ _						8.			
							-						8.			
							- - -						9.			
		Strikes		sing De	tails	Rei	marks									
ck at (m) Ca	asing to (m	Time (min) Rose to (	m) To (	m) [	iamet			pection pit	t excavated to 1.20m. ntered.							
						Ter	minatio	n Reason			Last Updated		\AG:			

		Project No.	Project Name:			Probe ID
	CALICEVAVAV	20-0399B	Bus Connects Route	6 Lucan to City Cent	re	
	CAUSEWAY GEOTECH	Coordinates	Client:	-		R6-
	——GEOTECH	706305.22 E	National Transport Au	uthority (NTA)		CP02DP
Method:		7	Client's Representat	tive:		Sheet 1 of 1
Dynamic Probing		735340.10 N	AECOM/Mott MacDo	nald		Scale: 1:50
Probe Type:		Elevation	Final Depth:	Date:	Operator:	
DPSH-B		52.79 mOD	3.05		JC	FINAL
Depth		'	Blows/100mm	•	•	Torque
(m)	10	20	30	4	0	(Nm)
_						
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_ 1						
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= 						
-  -						
_ 2						
-  -	9 10					
	9 9	_				
- 3		15			39	50
-						30
_						
<u>-</u>						
- 4						
- <b>4</b> -						
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<u>-</u>						
<u> </u>						
_						
8						
<u>-</u>						
_ -						
_ _ 9						
_						
E						
_						
Fall Height:	Remarks:					
750 mm	Follow on from R6-CP02.					
Hammer Mass:						
64 kg						
Cone Diameter: 51 mm						<b>W</b> AGS

	CAUSEW	ECH		20-(	ect No. 0399B	Project Client: Client's	National	nects Route 6 Lucar Transport Authorit 'Mott MacDonald			orehole R6-CP(	
Method Cable Percussion	Plant Used Dando 2000	<b>Top (m)</b> 0.00	4.00	7069	28.72 E 71.78 N	Final De	-	Start Date: 20/10/2 End Date: 20/10/2			heet 1 o Scale: 1: FINAL	:40
Depth Sample / (m) Tests	Field Records		Casing Water Depth Depth (m) (m)	Level mOD	Depth (m)	Legend	1	Description		Water	Backfill	
(m) Tests  .50 B5 .50 ES1  .00 B6 .00 ES2 .20 - 1.65 U12  .00 B7 .00 D10 .00 ES3 .00 - 2.45 SPT (S)  .00 B8 .00 D11 .00 ES4 .00 - 3.45 SPT (S)	Ublow=50 90%  N=10 (2,2/2,2,2,4) Ham 0643  N=10 (2,3/2,3,2,3) Ham 0643  Water strike at 3.00m  N=50 (25 for 25mm/50 50mm) Hammer SN = 0	nmer SN =	1.00 Dry 1.50 3.00 1.50 3.00	## ## ## ## ## ## ## ## ## ## ## ## ##	(m) - 0.10 - 0.40 - 1.00 - 1.20 - 1.20 - 1.20		coarse. Gravel is sul lithologies. MADE GROUND: Gr lithologies. Sand is in MADE GROUND: So coarse. Gravel is sul lithologies. Brown fine to coars Soft becoming firm fine to coarse. Grav mixed lithologies.	oft brown sandy gravelly bangular to subrounded rey sandy subrounded Gfine to coarse. oft brown sandy gravelly bangular to subrounded e SAND.  brown slightly gravelly s	fine to coarse of mixed  RAVEL of mixed  CLAY. Sand is fine to fine to coarse of mixed  andy silty CLAY. Sand is bunded fine to medium of the coarse of mixed fine to medium of the coarse of mixed.		Backfill	2.5 3.4 4.4 5.4 6.3
	n) Time (min) Rose to (n  Water Added				me (hh:mm) 01:00	Remarks Hand dug ii	nspection pit excavate	ed to 1.20m.				
							on Reason  on refusal.		<b>Last Updated</b> 16/12/2020	W	AC	_ }{

		CAUSEM	ECH			20-0	ct No. 399B	Client: National Transport Authority (NTA) Client's Rep: AECOM/Mott MacDonald						R6-CP04				
Metho Light Percu		Plant Used Dando Terrier	<b>Top (m)</b>	3.6		Coord	linates	Final De	<b>epth:</b> 3.60 m	21/10/2020	<b>Driller:</b> JC		Sheet 1 o Scale: 1:					
0							69.62 E 79.08 N	Elevatio	on: 51.36 mOD	End Date:	21/10/2020	Logger: SF		FINAL				
Depth (m)	Sample / Tests	Field Records		Casing Depth (m)	Water Depth (m)	Level mOD	Depth (m)	Legend	-	Des	cription	<del></del>	Water	Backfill				
.10 .10 - 1.20 .50	ES1 B1 ES2					51.26	0.10		TOPSOIL MADE GROUND: St content and concre subangular to subro	ete fragments.	Sand is fine to c	oarse. Gravel is			0.5			
.20 - 1.70 .20 - 1.65 .70 .70 - 2.00	B4 SPT (S)	N=36 (8,12/13,7,7,9) F SN = 0696 Ublow=127 80%	Hammer	0.00		49.66	1.70	, , , , , , , , , , , , , , , , , , ,	Stiff brown sandy g subrounded to sub-						1.5			
.50	D7						- - - -	(							2.5			
.00 .00 - 3.60 .00 - 3.45		N=4 (2,1/1,1,1,1) Ham 0696	mer SN =	0.00	3.30	48.36	3.00	( × × × × × × × × × × × × × × × × × × ×	Soft brown sandy g to coarse. Gravel is mixed lithologies.						3.0 -			
.60 - 3.78	SPT (C)	Water strike at 3.00m N=50 (19,25/50 for 30 Hammer SN = 0696	mm)	0.00	Dry	47.76	3.60	Č× ×.X		End of Bore	ehole at 3.60m				3.5			
							- - -								4.5			
							- - -								5.0 -			
							- - - -								5.5			
							- - -								6.0			
							-								6.5			
							-								7.0			
							-								7.5			
							-								8.0			
							-								8.5			
							-								9.0			
uck at (m) Ca		Strikes Time (min) Rose to (1) Time 20 Time 20		nsing C	<b>Detail</b> s		marks nd dug ins	spection pi	t excavated to 1.20m.					•				
								n Reason				Last Updated 16/12/2020		\AC				

	<i>y</i> –	GEOT	ECH			20-0	ct No. 399B	Project Client: Client's			R6-CP05				
Method Light Percus		Plant Used Dando Terrier	<b>Top (m)</b> 0.00	<b>Base</b> 3.5	_	Coordinates		Final De	<b>Start Date:</b> 20/10/202	<b>Driller:</b> JC		heet 1 o Scale: 1:			
J							0.65 E 5.16 N	Elevatio	n: 51.55 mOD <b>End Date:</b> 20/10/202	D Logger: CH		FINAL			
Depth (m)	Sample / Tests	Field Records		Casing Depth (m)	Water Depth (m)	Level mOD	Depth (m)	Legend	Description	<u>'</u>	Water	Backfill			
(m) 1.10 - 1.20 1.50 1.00 1.20 1.20 - 1.65 1.00 1.50 1.50 1.50 1.50 1.50 1.50 1.5	Tests B1 ES2 ES3 D4 SPT (S) U5 D6 ES6 B7 D9 ES8 SPT (S) SPT (C)	N=15 (3,3/2,4,5,4) Har 0696 Ublow=86 100% N=30 (4,6/5,4,9,12) Ha = 0696 Water strike at 3.10m N=50 (20,37/50 for 60 Hammer SN = 0696	nmer SN =	0.00 0.00	Dry Dry			Legend  The state of the state	Description  TOPSOIL  MADE GROUND: Firm becoming stiff brown saligh tobble content. Sand is fine to coarse. Grasubrounded fine to coarse of mixed lithologies subrounded of mixed lithologies.  Stiff becoming very stiff brown slightly sandy si Sand is fine to coarse. Gravel is subangular to scoarse of mixed lithologies.  End of Borehole at 3.52r	vel is subangular to Cobbles are ightly gravelly CLAY. ubrounded fine to		Backfill	0.5 1.0 1.5 2.0 3.0 4.5 5.0 6.0 6.5 7.0 8.0 8.5		
							<u> </u>						9.0		
		Strikes ) Time (min) Rose to ( 20 2.90		sing D	<b>etails</b>		marks and dug ins	spection pi	excavated to 1.20m.		•				
						Ter	minatio	n Reason		Last Updated 16/12/2020		AG	_		

		GEOT	ECH	T		20-0	ct No. 399B	Client: National Transport Authority (NTA) Client's Rep: AECOM/Mott MacDonald							R6-CP06				
Metho Light Percu		Plant Used Dando Terrier	<b>Top (m)</b>	Base 2.0			4.30 E	Final De	<b>epth:</b> 2.60 m	Start Date:	21/10/2020	Driller:	JC		heet 1 c Scale: 1:				
							8.90 N	Elevatio	on: 51.61 mOD	End Date:	21/10/2020	Logger:	СН		FINAI	L			
Depth (m) 10 - 0.40	Sample / Tests	Field Records	;	Casing Depth (m)	Water Depth (m)	Level mOD 51.51	Depth (m)	Legend	TOPSOIL	Des	cription			Water	Backfill				
10 - 0.40	PI					31.31	0.10		MADE GROUND: Gr GRAVEL of mixed lit		_		arse	1					
50 60 - 1.20	ES2 B3					51.01	0.60		MADE GROUND: Lig				<b>Υ</b> Α			0.5			
							[		GRAVEL of mixed lit		iai to subangulai	mic to coai	30			1.0			
00 20 - 1.80	ES4 B5					50.41	1.20		MADE GROUND: Lo	ose light grev	angular to suba	ngular fine t	·n			1.0			
20 - 1.65		N=6 (1,1/2,1,2,1) Ham 0696	mer SN =	0.00	Dry		-		coarse GRAVEL of m		-	rigulai iliic t				1.5			
	200					49.81	1.80		Very stiff brown sar	ndy gravally C	LAV with low coh	hle content	Sand is						
90 90	D6 ES6						-		fine to coarse. Grav mixed lithologies. C	el is subangu	lar to subrounde	d fine to coa	rse of			2.0			
00 00	1	Ublow=140 100%		0.00	Dry		-		lithologies.			Sanaca Of I	u			1			
50 60 - 2.85	D8 SPT (C)	N=50 (6,14/50 for 100	mm)	0.00	Dry	49.01	2.60	0 0 0 0		End of Bor	ehole at 2.60m					2.5			
		Hammer SN = 0696					_									3.0			
							[												
							F									3.5			
							-									4.0			
							E									4.5			
							-									4.3			
																5.0			
							-									5.5			
							-									6.0			
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							-									0.3			
							[ -									7.0			
							-									7.5			
							_									8.0			
							-									8.5			
							-									0.3			
							F									9.0			
																_			
ck at (m) Ca		Strikes ) Time (min) Rose to (		sing [	<b>Detail</b> Diam		marks	nection :	t everyated to 1 30										
u. (111) Ca	-38 60 (111	, () 1030 10 (1	, 10 (1	,	uIIII		na aug ins groundwa		t excavated to 1.20m. ntered.										
						Ter	mination	. Posco-			1	lact Had	ated						
						liei	ınınatıoı	ı keason				Last Upda	icea ,	-	AC				

			EC	OTI	ECI	Н	<b>In</b>	()	20-0	ect No.	Project Client: Client's			Authority (N7		re	1	oreho	P07
Meth Light Perc	cussion	Plant U Dando To	errie		0.	00	<b>Base</b> 4.0	00		dinates	Final De	<b>epth:</b> 10.70 m	Start Date:	24/09/2020	Driller:	JC+KW		Sheet 1 Scale:	
Rotary D Rotary C		Hanjin Hanjin			1	00 20	5.2 10.		706983.83 E 735121.30 N		Elevatio	on: 56.05 mOD <b>End Date</b> : 23/10/202			Logger:	CH+NP		FINA	٩L
Depth (m)	Sample / Tests	Fie	eld Re	cords			Casing Depth (m)	Water Depth (m)	Level mOD	Depth (m)	Legend		Des	cription			Water	Backfi	11
0.20 - 1.00 0.50 0.50	ES ES6							55.85 55.05	0.20		TOPSOIL  MADE GROUND: Fit coarse. Gravel is su lithologies.	bangular to su	brounded fine t	o coarse of	mixed			0.5 -	
1.00 1.00 - 2.00 1.20 - 1.65		N=5 (2,1/1,1, 0696	,1,2)	Hamr	mer S	N =	0.00	Dry				Soft brown slightly Gravel is subangula lithologies.				o coarse.			1.5 -
2.00 2.00 2.00 - 2.50 2.50 - 3.20	ES2 U8 B9 B10	Ublow=38 0% Seepage at 2.		1			0.00	Dry	53.55	2.50		Stiff grey slightly sa subangular to subro	, . ,				_		2.0 -
3.00 3.00 3.00 - 3.45 3.20 3.20 - 4.00	D11 ES3 SPT (S) ES12 B12	N=22 (3,3/5,6 0696	6,5,6	) Ham	nmer	SN =	0.00	Dry	52.85	3.20		Stiff light brown silt coarse. Gravel is su lithologies.							3.0 -
4.00	ES4								52.05	4.00		Very stiff brown ver (Driller's descriptio		ly CLAY with hig	gh cobble co	ontent.			4.0 -
5.20 5.20	C11 ES		100			NI			50.85 50.55	5.20 (0.30) 5.50	* * * * * * * * * * * * * * * * * * *	Brown slightly silty limestone. Sand is f Firm brown sandy g subangular fine to d	ine to coarse. gravelly SILT. Sa	and is fine to coa					5.5 -
6.20 6.20	C11					NI			49.85	6.20	* * * * * * * * * * * * * * * * * * *	Brown and grey sar mixed lithologies w Cobbles are subang	ith medium co	bble content. S					6.5
6.85	С		100	23	7	NI NI			49.20 48.95	6.85 (0.25) 7.10 (0.50)	****	Medium strong (loo weathered: slightly silty gravelly sand d Discontinuities: 1. 0 to 10 degree bo	reduced stren leposits.	gth, closer fract	ture spacing	g with			7.0 -
7.60 7.70 7.80	С					5			48.45	7.60		undulating, rough v between fracture s Brown silty gravelly limestone. Medium strong thin	urfaces. fine to coarse	SAND. Gravel is	subangula	r of			8.0 —
8.50 8.85 - 9.05	С		93	93	75							unweathered: close Discontinuities: 1. 0 to 10 degree be planar and slightly	edding fracture	es, closely space	ed (40/180/	'280),			8.5 -
9.20									47.00	9.05		Medium strong thin			Partially wea	athered:			9.0
	Water	Strikes	iCR	SCR	RQD	FI	Chise	elling	g Detail	s	Remarks								
Casing D	Casing to (m 2.50 Details Diam (mm)	) Time (min)  Water	Add		m) F			To (		ne (hh:mm)	Hand dug i No ground	nspection pit excavat water encountered. extended by dynami							
5.20	200						Barro K6L	el				on Reason I at scheduled depth.			<b>Last Upo</b>		W	A	GS

Method	CAUS					Base	(m)	20-0	ct No. 399B	Client: Client's	Rep: AECOM/	Transport A	Authority (N7			R6-CP07
Light Percussion Rotary Drilling Rotary Coring		Terrie n D8		0.0	00 00	4.0 5.2 10.7	00 10	70698	33.83 E 21.30 N	Final De Elevatio			24/09/2020	Driller: JC+KV  Logger: CH+N	/	Scale: 1:50
Depth (m) Sam	ples / Field Records	TCR	SCR	RQD	FI	Casing Depth (m)	Water Depth (m)	Level mOD	Depth (m)	Legend		Desc	cription		Water	Backfill
9.40 C 10.60 C 10.70		100	100	0	10			45.35	10.70		Medium strong thir slightly reduced stressandy silt deposits. Discontinuities: 1. 0 to 10 degree be planar and undulati joint surfaces. 2. At 10.35m to 10.4 clean. 3. At 9.05m to 10.33 with localised grey.	ength, closer for edding fracture ing, smooth wi 45m: 25 to 75 5m: 80 to 90 d	es, closely space ith grey sandy s degree joint, un	with localised ge ed (30/150/390), ilt deposits on som indulating, smooth, dulating, smooth		9.5 -
Struck at (m) Casing to 2.50  Casing Details		Rose					elling To (m	Details n) Tim	e (hh:mm)	No groundy	nspection pit excavate vater encountered. extended by dynamic	End of Borel	hole at 10.70m	rface at 9.20m to		11.0 —  11.5 —  12.0 —  12.5 —  13.0 —  14.5 —  14.5 —  15.0 —  16.5 —  17.0 —  17.5 —  18.0 —  18.5 —  18.5 —
To (m) Diam (r 5.20 200		То	(m)		Core	Barre	el	Flush	Туре	Termination	on Reason			Last Updated		
					SI	K6L		Polyr	mer	Terminated	at scheduled depth.			16/12/2020		AGS

200			Project Name:			Probe ID
	CALISEWAY	20-0399B	Bus Connects Route	6 Lucan to City Centr	е	R6-
+	CAUSEWAY GEOTECH	Coordinates	Client:			CP07DP
	GEOTECH	706983.83 E	National Transport Au	uthority (NTA)		CPU/DP
Method:			Client's Representa	tive:		Sheet 1 of 1
Dynamic Probing	}	735121.30 N	AECOM/Mott MacDo			Scale: 1:50
Probe Type:	<u> </u>	Elevation	Final Depth:	Date:	Operator:	554.51 1.55
DPSH-B		56.05 mOD	6.20		JC	FINAL
Depth (m)			Blows/100mm			Torque (Nm)
(111)	10	20	30	40	)	(MIII)
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Fall Height:	Remarks:	on				
750 mm	Follow on from R6-CP07 WS Secti	OII.				
Hammer Mass: 64 kg						
Cone Diameter:	-					111 400
51 mm						<b>\</b> \\AGS

		CAUSEW					ect No. 1399B	Project N Client: Client's R			Authority (NT		e		orehole	
<b>Methoc</b> Cable Percus		Plant Used Dando 2000	<b>Top (m)</b> 0.00	Base 4.2	_	70689	99.18 E 25.89 N	Final Dept			21/10/2020	Driller:			heet 1 o Scale: 1: FINAL	:40
Depth (m)	Sample / Tests	Field Records	;	Casing Depth (m)	Water Depth (m)	Level mOD	Depth (m)	Legend		Desc	ription		ı	Water	Backfill	Π
I	B5 ES1					51.90	- 0.40	li Li	MADE GROUND: So coarse. Gravel i sub- ithologies. cose brown gravel ine to medium of n	angular to sub	rounded fine to	coarse of r	nixed	-		0.5
00 20	B6 ES2 D10 SPT (S)	N=16 (2,3/2,3,4,7) Han 0643	mmer SN =	0.00	Dry	51.20	1.10		tiff brown sandy gr ubangular to subro							1.0
.00 .00	B7 ES3 ES7 U13	Ublow=50 100%		0.00	Dry	50.30	2.00		itiff greyish brown s s subangular to sub							2.0
.00	B8 D11 ES4 SPT (S)	N=23 (4,5/5,5,6,7) Han 0643	mmer SN =	0.00	Dry		-									3.0
.00	B9 D12 SPT (S)	N=50 (40 for 125mm/5 25mm) Hammer SN = 0		0.00	Dry	48.30 48.10	- 4.00 - 4.20		Grey sandy silty sub o coarse. (Possible	bedrock)	e GRAVEL of lim	estone. Sar	nd is fine			4.0
							- - - - - - -									5.0
							- - - - - - -									6.0
							- - - - - -									7.0
							-									
Casing De	ing to (m	r Strikes  i) Time (min) Rose to (r  Water Added	4.00	(m)	elling To (I		ne (hh:mm) 01:00		pection pit excavate ter encountered.	ed to 1.20m.				ı		
To (m) Di	<u>ameter</u>	From (m) To (m)						<b>Termination</b> Terminated or				<b>Last Upd</b> 16/12/2	lated 020	W	AC	

	<u> </u>	CAUSEW	ECH			-	ect No. 0399B	Project Client: Client's			Authority (N			Borehol R6-CP	
Method Cable Percuss	sion	Plant Used Dando 2000	<b>Top (m)</b> 0.00	_	e (m) 10		dinates 36.68 E	Final De	<b>pth:</b> 5.10 m	Start Date:	23/10/2020	<b>Driller:</b> BN	М	Sheet 1 Scale: 1	
						735065.33 N		<b>Elevation:</b> 45.94 mOD <b>End Date:</b> 23/10/202				Logger: CH	4	FINA	١L
	ample / Tests	Field Records	i	Casing Depth (m)	Water Depth (m)	Level mOD	Depth (m)	Legend	TOPSOIL	Desc	cription			Backfil	
.50 E	35 351					45.64	0.30		MADE GROUND: So to coarse. Gravel is mixed lithologies.						0.5
.00 E. .00 E. .20 D		N=2 (3,0/0,1,0,1) Ham 0696	mer SN =	0.00	Dry	44.74	1.20		MADE GROUND: Ve fine to coarse. Grav mixed lithologies.						1.5
00 D		N=2 (0,1/0,1,0,1) Ham 0696	mer SN =	0.00	Dry		-								2.0
00 E	88 554 558 J15	Ublow=15 80%		0.00	Dry	42.94	3.00		Very soft brown sar subangular to subro				vel is		3.0
00 D		N=25 (4,4/5,6,7,7) Har 0696	mmer SN =	0.00	Dry	41.94	- 4.00		Stiff brown sandy g subangular to subro						4.0
.00 D		N=50 (34 for 125mm/5 21mm) Hammer SN =		0.00	Dry	41.14 40.84	5.10		Grey sandy angular coarse. (Possible be	edrock)	EL of limestone.	Sand is fine to			5.0
							-								6.5
cuck at (m) Casin	ng to (m	Strikes   Time (min)   Rose to (iiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiiii	4.9	(m)	To (		me (hh:mm) 01:00		nspection pit excavat water encountered.	ed to 1.20m.			•		•
									on Reason on refusal.			Last Update 16/12/2020	ed	\\A(	GS

		CAUSEW					ct No. 399B	Project Client: Client's			Authority (N1				rehole	
Metho Cable Perc		Plant Used Dando 2000	<b>Top (m)</b> 0.00	4.10	)	70999	6.96 E		Final Depth: 4.10 m S  Elevation: 25.45 mOD E		24/10/2020			Sheet 1 of Scale: 1:40 FINAL		
Depth (m)	Sample / Tests	Field Records	•	Casing V Depth (m)		Level mOD	Depth (m)	Legend Description				<u> </u>		Water	Backfill	
.50 .50 .00 .00 .00	ES2  B4 ES ES5								MADE GROUND: BI of limestone. Sand  MADE GROUND: Fit gravelly CLAY. Sand subrounded fine to	rm becoming s	se. stiff brown sligh se. Gravel is sub	tly sandy sligl angular to				1.0
20 20 - 1.65 00 00 00 00 00 - 2.45	B7 D6 ES8	N=9 (6,4/2,2,2,3) Hami 0643 N=20 (4,4/5,4,5,6) Han 0643														2.0
00 00 00 00 - 3.45	B10 D9 ES11 SPT (C)	N=39 (6,7/9,9,10,11) H SN = 0643	lammer	1.50 [	Dry	2.45	- 3.00 		MADE GROUND: Ve coarse. Gravel is sul lithologies.							3.5
.00 - 4.08	SPI (C)	N=50 (25 for 50mm/50 25mm) Hammer SN = 0		1.50 (	iry 2	1.35	4.10			End of Bore	ehole at 4.10m					5.5.5 5.5.6 6.6
Casing D	asing to (m	r Strikes  i) Time (min) Rose to (r  Water Added  From (m) To (m)	n) From 3.90	m)	lling D To (m) 4.10	Tim	e (hh:mm)		nspection pit excavat water encountered.	ed to 1.20m.						7.0
	_00								on Reason			<b>Last Upda</b> 16/12/202	ted		AC	

Method	CAUSEW GEOT	AY ECH	Page 1	(m)	20-0	ect No.	Project Client: Client's	National	nects Route 6 Luc Transport Author 'Mott MacDonald	rity (NTA)	Centre		orehole	1
Cable Percussion	Dando 2000	0.00	4.20		71000	08.77 E 73.64 N	Final De	-	<b>Start Date:</b> 24/10 <b>End Date:</b> 24/10		riller: BM ogger: GH		Sheet 1 of Scale: 1:4 FINAL	10
Depth Sample (m) Tests	/ Field Records		Depth D	Vater Depth (m)	Level mOD	Depth (m)	Legend		Description			Water	Backfill	
(m) Tests  .50 B2 .50 ES1 .00 B5 .00 ES4 .20 D3 .20 ES3 .20 - 1.65 SPT (S)  .00 B8 .00 D6 .00 ES7 .00 - 2.45 SPT (S)  .00 B10 .00 D11 .00 ES11 .00 ES9 .00 - 3.45 SPT (S)  .00 B12 .00 - 4.12 SPT (S)	N=10 (4,3/2,3,2,3) Han 0643  N=21 (4,4/5,5,5,6) Han 0643  N=45 (7,8/10,10,12,13 SN = 0643  N=50 (41 for 100mm/5 25mm) Hammer SN = 0	nmer SN =  ) Hammer  50 for  0643	1.50 [ 1.50 ]	Dry Dry Dry	24.83 24.03 22.13	(m) - 0.30	Remarks	MADE GROUND: Fir gravelly CLAY. Sand subrounded fine to  Possible MADE GRC is fine to coarse. Gramixed lithologies.	ack sandy angular fine ine to coarse.  The becoming stiff brow is fine to coarse. Graw medium of mixed lith and the substitution of the substituti	e to coarse G own slightly s vel is subang hologies.	andy slightly ular to	,	Backfill	0.5 - 1.0 - 1.5 - 2.0 - 3.0 - 3.5 - 4.0 - 6.5 - 7.0 -
Casing Details To (m) Diameter 1.50 200	Water Added	4.10		4.20		01:00	No ground Terminati	on Reason  on refusal.	eu to 1.20m.		ast Updateo 16/12/2020	3	\ <b>A</b> G	iS

	GEOT	ECH		20-0	ect No. 1399B	Project Client: Client's			Authority (NT		ı	Borehole ID
Method Light Percussion	Plant Used Dando Terrier	0.00 Ba	1.40	70532	29.45 E 16.12 N	Final De			24/09/2020	Driller: JC  Logger: CH		Sheet 1 of 1 Scale: 1:50 FINAL
Depth Sample /	Field Records	Ca De	sing Water	Level	Depth	Legend	33.43 11100		cription	LOGGCI. CIT	ater	
(m) Tests  1.20 D1  1.40 ES2  1.40 - 1.40 B2  1.50 ES4  1.00 ES5  1.20 D3  1.20 - 1.58 SPT (S)	N=50 (8,11/50 for 235) Hammer SN = 0696	D.	puth my (m)	53.05 52.05	1.40	Legend	TOPSOIL  Very dense brown v coarse GRAVEL of n	very sandy silt nixed lithologi	y subangular to		Water Water	Backfill
	Strikes   Time (min)   Rose to (r		g Detail Diam	eter Ha		spection pit ater encour	excavated to 1.20m.					1 7
					rminatio	n Reason				Last Updated 16/12/2020	W	\AG\$

Light Percussion   Dando Terrier   0.00   0.87   705693.37   Final Depth: 0.87 m   Start Date: 24/09/2020   Origine: JC   Scalie: 150			OTECH	1		Project	399B	Project Client: Client's			Authority (NT		e	F	orehole	02
The state of the control of the cont	Method Light Percussic				87	70563	5.37 E								Scale: 1:	50
Section   Sect			ecords	Casing Depth	Water	Level	Depth		H. 64.42 IIIOD	ļ	1	Logger.	СП	ater		- 
Nater Strikes  Casing Details  Remarks  Last Jordane In Control Contro	.30 ES1 .30 - 0.87 B1 .50 ES2	2			(m)	64.12	0.30		Very dense brown v	very sandy ver VEL of mixed li	y silty subangula thologies. Sand			<u> </u>		0.5
Termination Reason Last Updated		/ater Strikes				er <sub>Han</sub>	d dug ins									6.0
						Terr	minatio	ı Reason				Last Upd	ated	I		



## APPENDIX C CORE PHOTOGRAPHS





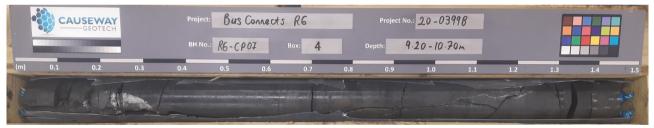
R6-CP07 Box 1 (5.20-6.20m)



R6-CP07 Box 2 (6.20-7.70m)



R6-CP07 Box 3 (7.70-9.20m)



R6-CP07 Box 4 (9.20-10.70m)





## APPENDIX D SLIT TRENCH LOGS AND SKETCHES



			Proj	ect No.	Project	: Name:		1	rial Pit ID
S A	CALIS	SEWAY	20-	0399B		nnects Route 6 Lucan to City Centre			
		GEOTECH	Coor	dinates	Client:			]	R6-TP01
			7100	19.80 E	1	al Transport Authority (NTA)			
Method: Slit Trenching				62.84 N	1	s Representative: //Mott MacDonald			neet 1 of 1
Plant:			Flo	vation	Date:	I/INOLL INIACDONAID	Logger:	5	icale: 1:25
3t Tracked Exca	avator			5 mOD	26/10/	2020	GH		FINAL
Depth	Sample /	Field Records	Level	Depth	Legend	Description	911	Water	
(m)	Tests	rieid Records	(mOD)	(m)	Legena	CONCRETE		Wa	
			19.35 19.27	0.10		MADE GROUND: Grey slightly sandy angular fine to	coarse GRAVEL of		-
			19.27	0.18		\limestone. Sand is fine to coarse. MADE GROUND: Dark grey slightly sandy angular fin	e to coarse GRAVEI	-1	
				-		of limestone. Sand is fine to coarse.			
0.50	B1		18.97	0.48		MADE GROUND: Brown slightly sandy clayey angula	r fine to coarse		0.5 —
		Slow seepage at 0.65m.		-		GRAVEL of limestone. Sand is fine to coarse.		•	
				-					
				-					
				-					1.0
			18.35	1.10	******	End of trial pit at 1.10m			-
				-					<u> </u>
				-					
				-					1.5 —
				-					-
				-					-
				-					2.0
				-					-
				-					_
				-					2.5
									-
				-					_
				- -					
				-					3.0
									-
				-					-
				-					
				-					3.5 —
				-					-
				<u> </u>					
				-					
				-					4.0
				<u>-</u>					-
				-					_
				-					4.5 —
				-					-
				-					
				<u>-</u>					
			$\vdash$					$\perp$	
	Strikes	<b>Depth:</b> 1.10	Rem			la ta ha datamain. U			
Struck at (m) 0.65	Remarks Slow seepag		Base	or roundat	ion unab	le to be determined to presences of services.			
0.05	0.65m.	Length: 1.55							
		Stability:	Term	nination Re	ason:		Last Updated		
		Stable	Term	ninated at s	cheduled	depth.	16/12/2020		AGS

JOB NUMBER: JOB NAME: LOCATION: Bus Connects Lucan to City Centre 20-0399B R6 - TP01 CLIENT: CREW: PLANT & EQUIPMENT CLIENTS REPRESENTATIVE: National Transport authority (NTA) **AECOM** GH 3 Tonne Excavator & Hand Tools TRENCH: (SECTION & PLAN) Datum **TRENCH - ORIENTATION** 4 345° <sub>90°</sub>E 225 SW 180° Concrete S foundation TRENCH ORIENTATED: 345° FROM NORTH COORDINATES: DATUM WALL EASTING: 710019.80 NORTHING: 734262.84 ELEVATION: 19.45MDD 1550 Type of Service: Distance to Centre of Service (m) Diameter (in mm) Depth to Top TRENCH LENGTH (m): 1.55 **Details/Comments** No: of Service (m) TRENCH DEPTH (m): 1.10 01 100 0.56 0.21 <u>Jnknown</u> 100mm Black Duct Unknown 02 TRENCH WIDTH (m): 0.60 152 0.62 0.29 Unknown 152mm Yellow Duct Unknown 03 0.89 Unknown 100 0.57 <u> 100mm Black Duct Unknowr</u> STABILITY: STABLE 04 <u>Unknown</u> 100 0.73 0.76 100mm Black Duct Unknown 05 0.35 Unknown 100 1.05 - 1.16 2x100mm Black Duct Unknown GROUNDWATER: SLOW FLOW AT 0.65 06 07 SCALE: NTS@A3 80 09 DRAWN: BS 10 СН DATE EXCAVATED: 11 26-10-2020 12 13 14 15



## APPENDIX E SLIT TRENCH PHOTOGRAPHS





**R6-TP01** 



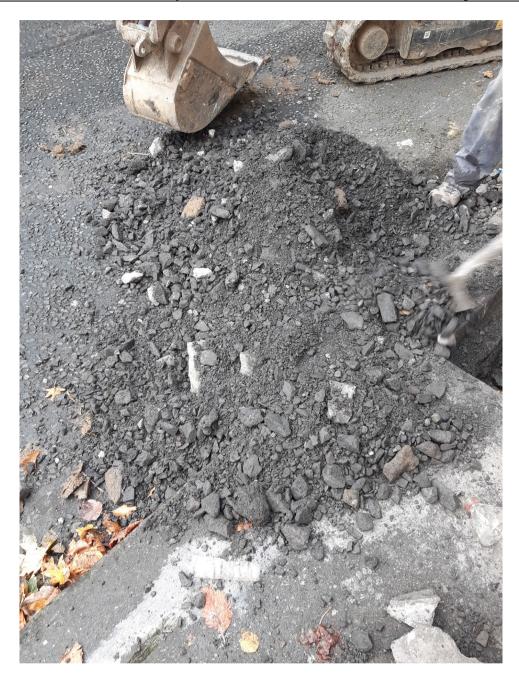


**R6-TP01** 



**R6-TP01** 





**R6-TP01** 





**R6-TP01** 





# APPENDIX F GEOTECHNICAL LABORATORY TEST RESULTS





## **HEAD OFFICE**

Registered in Northern Ireland. Company Number: NI610766

#### **REGIONAL OFFICE** Causeway Geotech (IRL) Ltd

Unit 3 Balbriggan Business Park, Balbriggan Co Dublin, Ireland, K32 EH36 ROI: +353 (0)1 526 7465

> Registered in Ireland. Company Number: 633786

www.causewaygeotech.com

#### **SOIL AND ROCK SAMPLE ANALYSIS** LABORATORY TEST REPORT

19 November 2020

<b>Project Name:</b>	Bus Connects - Route 6 - Lucan to City Centre
Project No.:	20-0399В
Client:	National Transport Authority (NTA)
Engineer:	AECOM/Mott MacDonald

We are pleased to attach the results of laboratory testing carried out for the above project. This memo and its attachments constitute a report of the results of tests as detailed in the Contents page(s).

The attached results complete the testing requested and we would therefore wish to confirm that samples will be retained without charge for a period of 28 days from the above date after which they will be appropriately disposed of unless we receive written instructions to the contrary prior to that date.

We trust our report meets with your approval but if you have any queries or require additional information, please do not hesitate to contact the undersigned.

Stephen Watson

**Laboratory Manager** 

Signed for and on behalf of Causeway Geotech Ltd















**Project Name:** Bus Connects - Route 6 - Lucan to City Centre

**Report Reference:** Schedule 1

The table below details the tests carried out, the specifications used, and the number of tests included in this report.

Tests marked with\* in this report are not United Kingdom Accreditation Service (UKAS) accredited and are not included in Causeway Geotech Limited's scope of UKAS Accreditation Schedule of Tests. Opinions and interpretations expressed herein are outside the scope of UKAS accreditation.

Material tested	Type of test/Properties measured/Range of measurement	Standard specifications	No. of results included in the report
SOIL	Moisture Content of Soil	BS 1377-2: 1990: Cl 3.2	37
SOIL	Liquid and Plastic Limits of soil-1 point cone penetrometer method	BS 1377-2: 1990: Cl 4.4, 5.3 & 5.4	12
SOIL	Particle size distribution - wet sieving	BS 1377-2: 1990: Cl 9.2	14
SOIL	Particle size distribution - sedimentation hydrometer method	BS 1377-2: 1990: Cl 9.5	11
SOIL	Moisture Condition Value at natural moisture content	BS 1377-4: 1990: Cl 5.4	1
SOIL	Undrained shear strength – triaxial compression without measurement of pore pressure (loads from 0.12 to 24 kN)	BS 1377-7: 1990: Cl 8	1
ROCK	Point load index	ISRM Commission on Testing Methods. Suggested Method for Determining Point Load Strength 1985	6
ROCK	Uniaxial Compressive Strength (UCS)*	ISRM Suggested Methods -Rock Characterization Testing and Monitoring, Ed. E T Brown - 1981	1

### **SUB-CONTRACTED TESTS**

In agreement with Client, the following tests were conducted by an approved sub-contractor. All sub-contracting laboratories used are UKAS accredited.

Material tested	Type of test/Properties measured/Range of measurement	Standard specifications	No. of results included in the report
SOIL – Subcontracted to Eurofins Chemtest Ltd (UKAS 2183)	pH Value of Soil		17
SOIL – Subcontracted to Eurofins Chemtest Ltd (UKAS 2183)	Sulphate Content water extract		17



Project No.

Project Name

20-0399B

Bus Connects Route 6 - Lucan to City Centre

		0	1 -		1	I						T		
Hole No.	Ref	Top	nple Base	Туре	Soil Description	Densi bulk	ity dry	W	Passing 425µm	LL	PL		Particle density	Casagrande Classification
				. ,		Mg/m	3	%	%	%	%	%	Mg/m3	
R6-CP01	6	1.00		В	Brown slightly sandy silty CLAY.			12.0						
R6-CP01	7	2.00		В	Brown slightly sandy slightly gravelly silty CLAY.			15.0	70	28 -1pt	19	9		CL
R6-CP01	14	3.00		U	Brown slightly sandy silty CLAY.			15.0						
R6-CP01	12	4.00		D	Brown slightly sandy silty CLAY.			16.0						
R6-CP01	13	5.00		D	Brown slightly sandy silty CLAY.			9.0						
R6-CP02	2	0.70		В	Brown slightly sandy silty CLAY.			24.0						
R6-CP02	4	1.20		В	Brown sandy slightly gravelly silty CLAY.			17.0	83	37 -1pt	21	16		CI
R6-CP03	12	1.20		U	Brown sandy gravelly silty CLAY.			33.0						
R6-CP03	7	2.00		В	Brown sandy gravelly silty CLAY.			16.0	60	30 -1pt	20	10		CL
R6-CP03	11	3.00		D	Brown sandy slightly gravelly silty CLAY.			15.0						
R6-CP04	4	1.20		В	Brown sandy slightly gravelly silty CLAY.			15.0						
R6-CP04	6	2.00		U	Brown sandy slightly gravelly silty CLAY.			14.0						
		-							-				ا ۸۵	01P Version 4

All tests performed in accordance with BS1377:1990 unless specified otherwise

LAB 01R Version 4

Key

Density test Liquid Limit Particle density

Linear measurement unless : 4pt cone unless : sp - small pyknometer

1pt - single point test

wd - water displacement
wi - immersion in water

cas - Casagrande method

gj - gas jar

Date Printed

Approved By

19/11/2020



Stephen.Watson



Project No.

Project Name

20-0399B

Bus Connects Route 6 - Lucan to City Centre

R6-CP04	Ref 9	Top 3.00	Base	Туре	Soil Description	Densi bulk Mg/m	dry	w %	Passing 425µm %	LL %	PL %		Particle density	Casagrande Classification
+		3.00		В		Mg/m	3	%						
R6-CP05	4				Brown sandy slightly gravelly silty CLAY.			17.0	70	70	76	%	Mg/m3	
		1.20		D	Brown sandy slightly gravelly silty CLAY.			16.0						
R6-CP05	5	2.00		U	Brown sandy gravelly silty CLAY.			14.0						
R6-CP05	6	2.50		D	Brown sandy gravelly silty CLAY.			12.0						
R6-CP05	7	2.60		В	Brown sandy gravelly silty CLAY.			9.6	47	28 -1pt	18	10		CL
R6-CP06	9	2.00		U	Brown sandy gravelly silty CLAY.			12.0						
R6-CP07	5	0.20		В	Brown sandy slightly gravelly silty CLAY.			18.0	72	41 -1pt	23	18		CI
R6-CP07	7	1.00		В	Brown sandy slightly gravelly silty CLAY.			18.0	63	40 -1pt	25	15		MI/CI
R6-CP07	11	3.00		D	Brown sandy slightly gravelly silty CLAY.			22.0						
R6-CP07	11	5.20		С	Brown sandy gravelly silty CLAY.			13.0	48	28 -1pt	13	15		CL
R6-CP07	11	6.20		С	Brown sandy very gravelly silty CLAY with coobles.			8.5						
R6-CP08	10	1.20		D	Brown sandy slightly gravelly silty CLAY.			17.0						

All tests performed in accordance with BS1377:1990 unless specified otherwise

LAB 01R Version 4

Key

Density test Liquid Limit Particle density

Linear measurement unless : 4pt cone unless : sp - small pyknometer

1pt - single point test

wd - water displacement

wi - immersion in water

cas - Casagrande method

gj - gas jar

Date Printed

19/11/2020

Approved By

U KAS
TESTING
10122

Stephen.Watson



Project No.

Project Name

20-0399B

Bus Connects Route 6 - Lucan to City Centre

Hole No.			mple		Soil Description	Densit bulk	ty dry	W	Passing 425µm	LL	PL	PI	Particle density	Casagrande
11010110.	Ref	Тор	Base	Туре	Con Booonphon	Mg/m3	-	%	%	%	%	%	Mg/m3	Classification
R6-CP08	7	2.00		В	Brown sandy slightly gravelly silty CLAY.			16.0	59	29 -1pt	19	10		CL
R6-CP08	11	3.00		D	Brown sandy gravelly silty CLAY.			17.0						
R6-CP08	12	4.00		D	Greyish brown sandy very gravelly silty CLAY.			7.9						
R6-CP09	11	1.20		D	Greyish brown sandy slightly gravelly silty CLAY.			25.0						
R6-CP09	12	2.00		D	Greyish brown sandy silty CLAY.			26.0						
R6-CP09	7	2.00		В	Greyish brown sandy silty CLAY.			26.0	50	32 -1pt	16	16		CL
R6-CP09	15	3.00		U	Greyish brown sandy silty CLAY.			27.0						
R6-CP10	4	1.00		В	Greyish brown sandy gravelly silty CLAY.			12.0	48	31 -1pt	17	14		CL
R6-CP10	6	2.00		D	Greyish brown sandy gravelly silty CLAY.			10.0						
R6-CP10	10	3.00		В	Greyish brown sandy gravelly silty CLAY.			12.0	50	28 -1pt	17	11		CL
R6-CP11	3	1.20		D	Greyish brown sandy slightly gravelly silty CLAY.			15.0						
R6-CP11	8	2.00		В	Greyish brown sandy slightly gravelly silty CLAY.			14.0	60	30 -1pt	15	15		CL
All to ata manfan	mad:	n 0000 ===	lanaa .::	h DC4	277:1000 unloss specified	oth omuic c							ΙΔF	3 01R Version 4

All tests performed in accordance with BS1377:1990 unless specified otherwise

LAB 01R Version 4

Key

Density test Liquid Limit Particle density

Linear measurement unless : 4pt cone unless : sp - small pyknometer

1pt - single point test

wd - water displacement

wi - immersion in water

cas - Casagrande method

gj - gas jar

Date Printed

19/11/2020

UKAS
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10122

Stephen.Watson

Approved By



Project No. Project Name

20-0399B

Bus Connects Route 6 - Lucan to City Centre

Hole No.			mple	I	Soil Description	Dens bulk	ity dry	W	Passing 425µm	LL	PL	PI	Particle density	Casagrande Classification
Hole No.	Ref	Тор	Base	Туре	Soil Description	Mg/m		%	%	%	%	%	Mg/m3	Classification
R6-CP11	11	3.00		D	Greyish brown sandy slightly gravelly silty CLAY.			11.0						
														_
All tests perfor	I tests performed in accordance with BS1377:1990 unless specified otherwise LAB 01R Version 4													

Key Density test Liquid Limit

Particle density

gj - gas jar

Approved By

Linear measurement unless:

wi - immersion in water

4pt cone unless: sp - small pyknometer

19/11/2020

Date Printed

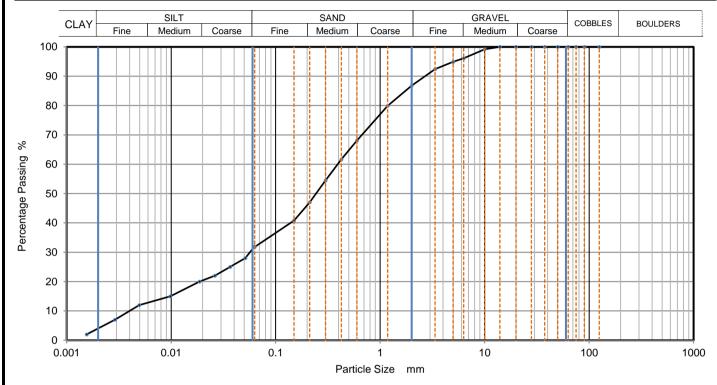
wd - water displacement cas - Casagrande method

1pt - single point test

Stephen.Watson



CAUSEWAY	DART	ICLE SIZE DIST	TDIDLITION	Job Ref	20-0399В
GEOTECH	PARI	ICLE SIZE DIST	IKIBOTION	Borehole/Pit No.	R6-CP01
Site Name	Bus Connects Route 6	6 - Lucan to City Ce	ntre	Sample No.	7
Soil Description	Brown slightly sandy slig	ghtly gravelly silty CLA	AY.	Depth, m	2.00
Specimen Reference	9	Specimen Depth	2 n	Sample Type	В
Test Method	BS1377:Part 2:1990, cla	uses 9.2 and 9.5		KeyLAB ID	Caus202010301



Sieving		Sedimentation		
Particle Size mm	% Passing	Particle Size mm	% Passing	
125	100	0.06300	32	
90	100	0.05127	28	
75	100	0.03668	25	
63	100	0.02624	22	
50	100	0.01866	20	
37.5	100	0.00980	15	
28	100	0.00495	12	
20	100	0.00290	7	
14	100	0.00155	2	
10	99			
6.3	96			
5	95			
3.35	92			
2	87			
1.18	80			
0.6	68	Particle density	(assumed)	
0.425	62	2.65	Mg/m3	
0.3	55			
0.212	47			
0.15	41			
0.063	32			

Sample Proportions	% dry mass		
Cobbles	0.0		
Gravel	13.2		
Sand	55.0		
Silt	28.1		
Clay	3.7		

Grading Analysis		
D100	mm	
D60	mm	0.392
D30	mm	0.0564
D10	mm	0.00413
Uniformity Coefficient		95
Curvature Coefficient		2

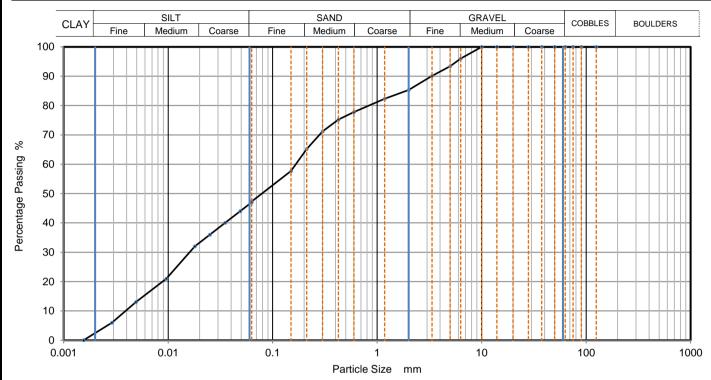
Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved

Stephen.Watson

CAUSEWAY	PARTICLE SIZE DISTRIBUTION			Job Ref	20-0399В	
GEOTECH	PAR	PARTICLE SIZE DISTRIBUTION			Borehole/Pit No.	R6-CP02
Site Name	Bus Connects Route	Bus Connects Route 6 - Lucan to City Centre			Sample No.	4
Soil Description	Brown sandy slightly gravelly silty CLAY.			Depth, m	1.20	
Specimen Reference	9 Specimen 1.2 m			Sample Type	В	
Test Method	BS1377:Part 2:1990, clauses 9.2 and 9.5			KeyLAB ID	Caus202010306	



Sieving		Sedim	Sedimentation			
Particle Size mm	% Passing	Particle Size mm	% Passing			
125	100	0.06300	47			
90	100	0.04889	44			
75	100	0.03502	40			
63	100	0.02508	36			
50	100	0.01795	32			
37.5	100	0.00960	21			
28	100	0.00491	13			
20	100	0.00290	6			
14	100	0.00155	0			
10	100					
6.3	96					
5	93					
3.35	90					
2	85					
1.18	82					
0.6	78	Particle density	(assumed)			
0.425	75	2.65	Mg/m3			
0.3	71					
0.212	65	1				
0.15	58	1				
0.063	47	1				

Dry Mass of sample, g	362
-----------------------	-----

Sample Proportions	% dry mass		
Cobbles	0.0		
Gravel	14.7		
Sand	38.0		
Silt	45.0		
Clay	2.3		

Grading Analysis		
D100	mm	
D60	mm	0.167
D30	mm	0.0159
D10	mm	0.00392
Uniformity Coefficient		43
Curvature Coefficient		0.39

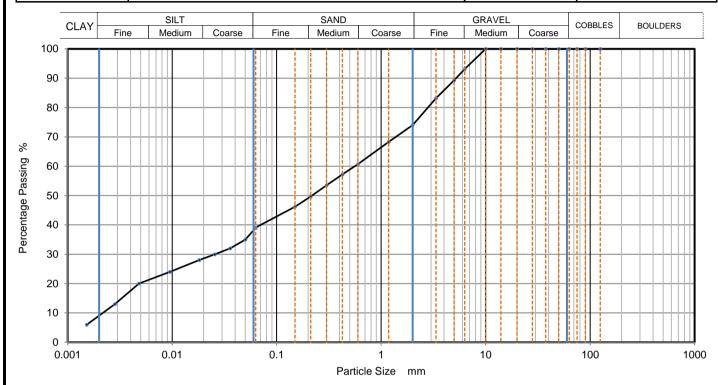
Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved

Stephen.Watson

CAUSEWAY	PARTICLE SIZE DISTRIBUTION			Job Ref	20-0399В	
—— GEOTECH	PANII	PARTICLE SIZE DISTRIBUTION			Borehole/Pit No.	R6-CP03
Site Name	Bus Connects Route 6 - Lucan to City Centre			Sample No.	7	
Soil Description	Brown sandy gravelly silty CLAY.			Depth, m	2.00	
Specimen Reference	9 Specimen 2 m Depth			Sample Type	В	
Test Method	BS1377:Part 2:1990, clauses 9.2 and 9.5			KeyLAB ID	Caus202010308	



		TT.		
Sieving		Sedimentation		
Particle Size mm	% Passing	Particle Size mm	% Passing	
125	100	0.06300	39	
90	100	0.05016	35	
75	100	0.03591	32	
63	100	0.02554	30	
50	100	0.01817	28	
37.5	100	0.00949	24	
28	100	0.00480	20	
20	100	0.00283	13	
14	100	0.00152	6	
10	100			
6.3	93			
5	89			
3.35	83			
2	74			
1.18	68			
0.6	61	Particle density	(assumed)	
0.425	57	2.65	Mg/m3	
0.3	54			
0.212	50			
0.15	46			
0.063	39			

436

Sample Proportions	% dry mass
Cobbles	0.0
Gravel	26.0
Sand	34.9
Silt	30.3
Clay	8.8

Grading Analysis		
D100	mm	
D60	mm	0.558
D30	mm	0.0265
D10	mm	0.0022
Uniformity Coefficient		250
Curvature Coefficient		0.57

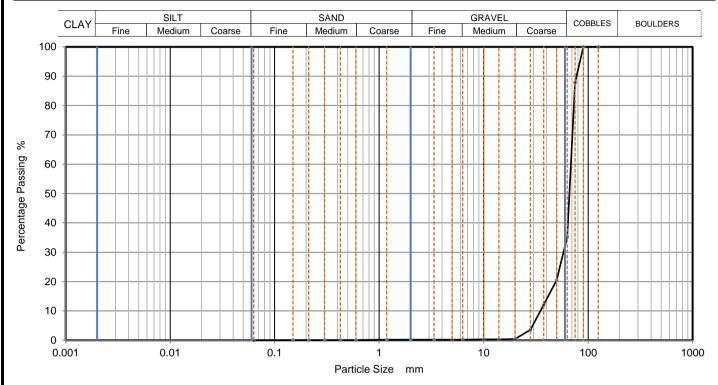
Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved

Stephen.Watson

CAUSEWAY	PARTICLE SIZE DISTRIBUTION		Job Ref	20-0399В	
—— GEOTECH	PANII	TICLE SIZE DISTRIBUTION		Borehole/Pit No.	R6-CP03
Site Name	Bus Connects Route 6	Bus Connects Route 6 - Lucan to City Centre			9
Soil Description	Brown subangular medium coarse GRAVEL with high cobble content.			Depth, m	4.00
Specimen Reference	3 Specimen 4 m			Sample Type	В
Test Method	d BS1377:Part 2:1990, clause 9.2			KeyLAB ID	Caus2020103010



Siev	ving	Sedime	ntation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	88		
63	35		
50	21		
37.5	12		
28	4		
20	0		
14	0		
10	0		
6.3	0		
5	0		
3.35	0		
2	0		
1.18	0		
0.6	0		
0.425	0	1	
0.3	0		
0.212	0	1	
0.15	0	1	
0.063	0		

Dry Mass of sample, g	4669
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Sample Proportions	% dry mass
Cobbles	64.7
Gravel	35.1
Sand	0.2
Fines < 0.063mm	0.0

Grading Analysis		
D100	mm	
D60	mm	68.4
D30	mm	58
D10	mm	34.9
Uniformity Coefficient		2
Curvature Coefficient		1.4

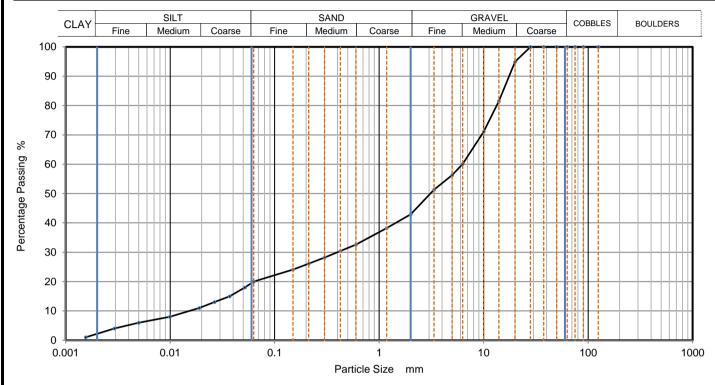
Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved

Stephen.Watson

CAUSEWAY	PARTICLE SIZE DISTRIBUTION -		Job Ref	20-0399В		
—— GEOTECH	PANI	ARTICLE SIZE DISTRIBUTION		Borehole/Pit No.	R6-CP05	
Site Name	Bus Connects Route 6	Bus Connects Route 6 - Lucan to City Centre			Sample No.	7
Soil Description	Brown sandy gravelly silty CLAY.			Depth, m	2.60	
Specimen Reference	9 Specimen 2.6 m			Sample Type	В	
Test Method	Method BS1377:Part 2:1990, clauses 9.2 and 9.5			KeyLAB ID	Caus2020103017	



Sieving		Sedimentation		
Particle Size mm	% Passing	Particle Size mm	% Passing	
125	100	0.06300	20	
90	100	0.05188	18	
75	100	0.03711	15	
63	100	0.02654	13	
50	100	0.01897	11	
37.5	100	0.00990	8	
28	100	0.00501	6	
20	95	0.00292	4	
14	82	0.00155	1	
10	71			
6.3	60			
5	56			
3.35	51			
2	43			
1.18	38			
0.6	33	Particle density	(assumed)	
0.425	30	2.65	Mg/m3	
0.3	28			
0.212	26			
0.15	24			
0.063	20			

Dry Mass of sample, g	2250
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Sample Proportions	% dry mass
Cobbles	0.0
Gravel	57.1
Sand	23.0
Silt	17.8
Clay	2.1

Grading Analysis		
D100	mm	
D60	mm	6.25
D30	mm	0.398
D10	mm	0.0161
Uniformity Coefficient		390
Curvature Coefficient		1.6

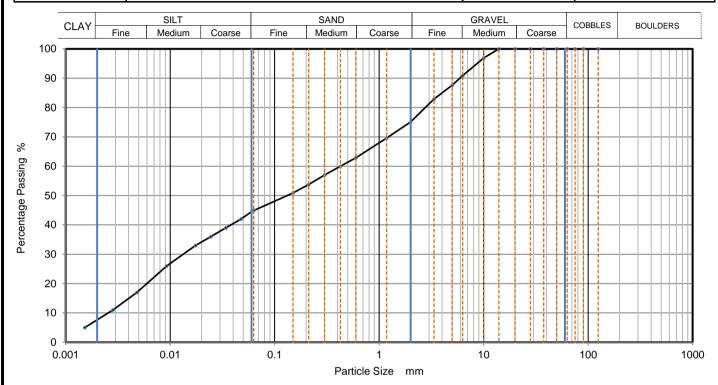
Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved

Stephen.Watson

CAUSEWAY	CALISEWAY DARTICLE SIZE DISTRIBUTION			Job Ref	20-0399В	
GEOTECH	PARII	PARTICLE SIZE DISTRIBUTION -			Borehole/Pit No.	R6-CP07
Site Name	Bus Connects Route 6	Bus Connects Route 6 - Lucan to City Centre			Sample No.	9
Soil Description	Brown sandy gravelly silty CLAY.			Depth, m	2.00	
Specimen Reference	3	Specimen 2 m Depth			Sample Type	В
Test Method	BS1377:Part 2:1990, clau	ses 9.2 and 9.5			KeyLAB ID	Caus2020103021



Sie	ving	Sedime	entation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.06300	45
90	100	0.04758	42
75	100	0.03411	39
63	100	0.02444	36
50	100	0.01751	33
37.5	100	0.00927	26
28	100	0.00480	17
20	100	0.00283	11
14	100	0.00152	5
10	97		
6.3	91		
5	88		
3.35	83		
2	75		
1.18	70		
0.6	63	Particle density	(assumed)
0.425	60	2.65	Mg/m3
0.3	57		
0.212	54		
0.15	51		
0.063	45		

Dry Mass of sample, g 537	Dry Mass of sample, g	537
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Sample Proportions	% dry mass		
Cobbles	0.0		
Gravel	24.9		
Sand	30.2		
Silt	37.5		
Clay	7.4		

Grading Analysis		
D100	mm	
D60	mm	0.425
D30	mm	0.0135
D10	mm	0.00261
Uniformity Coefficient		160
Curvature Coefficient		0.16

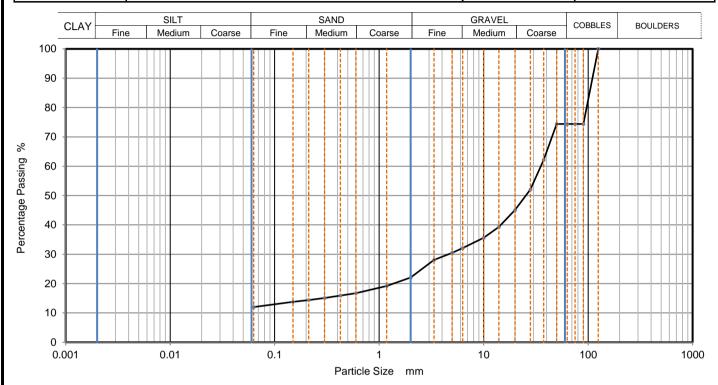
Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved

Stephen.Watson

CAUSEWAY PARTICLE SIZE DISTRIBUTION			Job Ref	20-0399В		
—— GEOTECH	PANII	PARTICLE SIZE DISTRIBUTION			Borehole/Pit No.	R6-CP07
Site Name	Bus Connects Route 6	Bus Connects Route 6 - Lucan to City Centre			Sample No.	11
Soil Description	Brown sandy very gravelly silty CLAY with coobles.			Depth, m	6.20	
Specimen Reference	8	8 Specimen 6.2 m			Sample Type	С
Test Method	BS1377:Part 2:1990, clau	se 9.2		·	KeyLAB ID	Caus2020103024



Siev	/ing	Sedime	ntation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	74		
75	74		
63	74		
50	74		
37.5	62		
28	52		
20	45		
14	39		
10	36		
6.3	32		
5	31		
3.35	28		
2	22		
1.18	19		
0.6	17		
0.425	16	1	
0.3	15		
0.212	14		
0.15	14		
0.063	12		

Dry Mass of sample, g	9944
, , , ,	

Sample Proportions	% dry mass
Cobbles	25.6
Gravel	52.4
Sand	10.1
Fines < 0.063mm	12.0

Grading Analysis		
D100	mm	125
D60	mm	35.2
D30	mm	4.59
D10	mm	
Uniformity Coefficient		
Curvature Coefficient		

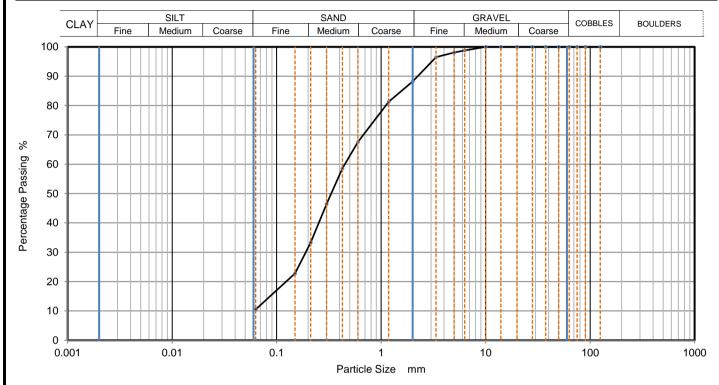
Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved

Stephen.Watson

CAUSEWAY PARTICLE SIZE DISTRIBUTION			Job Ref	20-0399В		
—— GEOTECH	GEOTECH PARTICLE SIZE DISTRIBUTION			Borehole/Pit No.	R6-CP08	
Site Name	Bus Connects Route 6	Bus Connects Route 6 - Lucan to City Centre			Sample No.	5
Soil Description	Brown slightly gravelly silty fine to coarse SAND.			Depth, m	0.50	
Specimen Reference	3 Specimen 0.5 m			Sample Type	В	
Test Method	BS1377:Part 2:1990, clau	3S1377:Part 2:1990, clause 9.2			KeyLAB ID	Caus2020103025



Siev	ving	Sedime	ntation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100		
90	100		
75	100		
63	100		
50	100		
37.5	100		
28	100		
20	100		
14	100		
10	100		
6.3	99		
5	98		
3.35	97		
2	88		
1.18	81		
0.6	68		
0.425	59	1	
0.3	46		
0.212	33	]	
0.15	23	]	
0.063	11		

Dry Mass of sample, g	358

Sample Proportions	% dry mass
Cobbles	0.0
Gravel	11.8
Sand	77.7
Fines < 0.063 mm	11.0

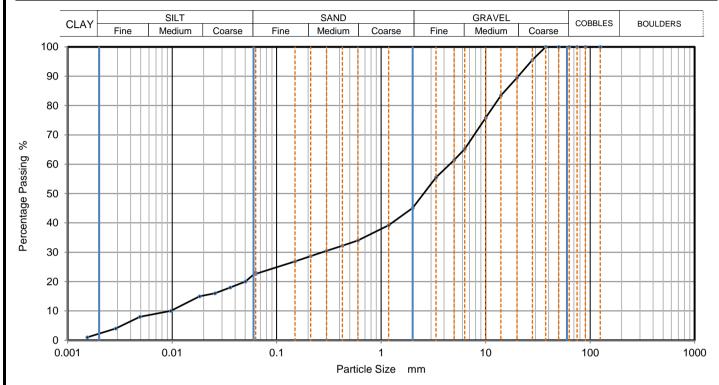
Grading Analysis		
D100	mm	
D60	mm	0.448
D30	mm	0.19
D10	mm	
Uniformity Coefficient		
Curvature Coefficient		

Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved	
Stephen.Watson	

CAUSEWAY	PARTICLE SIZE DISTRIBUTION		Job Ref	20-0399B		
GEOTECH	PARTICLE SIZE DISTRIBUTION			Borehole/Pit No.	R6-CP08	
Site Name	Bus Connects Route 6	Bus Connects Route 6 - Lucan to City Centre			Sample No.	8
Soil Description	Brown sandy slightly gravelly silty CLAY.			Depth, m	3.00	
Specimen Reference	Specimen 3 m Depth		Sample Type	В		
Test Method	3S1377:Part 2:1990, clauses 9.2 and 9.5			KeyLAB ID	Caus2020103029	



Sieving		Sedim	entation
Particle Size mm	% Passing	Particle Size mm	% Passing
125	100	0.06300	23
90	100	0.05016	20
75	100	0.03591	18
63	100	0.02570	16
50	100	0.01828	15
37.5	100	0.00971	10
28	96	0.00491	8
20	90	0.00288	4
14	83	0.00154	1
10	76		
6.3	65		
5	61		
3.35	56		
2	45		
1.18	39		
0.6	34	Particle density	(assumed)
0.425	32	2.65	Mg/m3
0.3	31		
0.212	29	7	
0.15	27	7	
0.063	23	7	

Dry Mass of sample, g	2219

Sample Proportions	% dry mass
Cobbles	0.0
Gravel	54.9
Sand	22.5
Silt	20.2
Clay	2.4

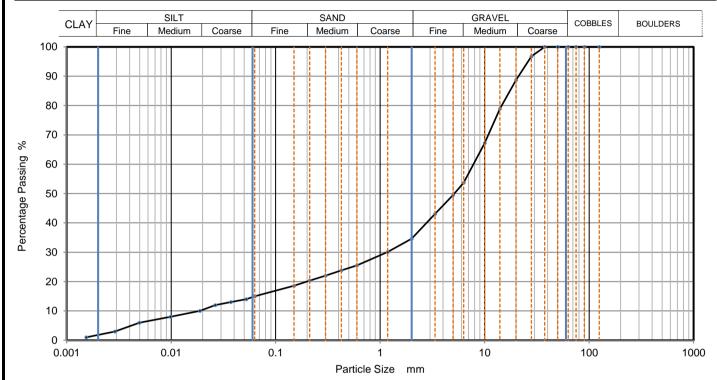
Grading Analysis		
D100	mm	
D60	mm	4.56
D30	mm	0.271
D10	mm	0.0101
Uniformity Coefficient		450
Curvature Coefficient		1.6

Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved Stephen.Watson

CAUSEWAY	PARTICLE SIZE DISTRIBUTION -			Job Ref	20-0399В
—— GEOTECH				Borehole/Pit No.	R6-CP08
Site Name	Bus Connects Route 6 - Lucan to City Centre			Sample No.	9
Soil Description	Greyish brown sandy very gravelly silty CLAY.			Depth, m	4.00
Specimen Reference	Specimen 4 m Depth			Sample Type	В
Test Method	3S1377:Part 2:1990, clauses 9.2 and 9.5			KeyLAB ID	Caus2020103032



Sieving		Sedimentation		
Particle Size mm	% Passing	Particle Size mm	% Passing	
125	100	0.06300	15	
90	100	0.05231	14	
75	100	0.03720	13	
63	100	0.02645	12	
50	100	0.01881	10	
37.5	100	0.00982	8	
28	97	0.00496	6	
20	89	0.00290	3	
14	79	0.00154	1	
10	67			
6.3	54			
5	50			
3.35	43			
2	35			
1.18	30			
0.6	26	Particle density	(assumed)	
0.425	24	2.65	Mg/m3	
0.3	22			
0.212	20			
0.15	19			
0.063	15			

Dry Mass of sample, g	2545
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Sample Proportions	% dry mass
Cobbles	0.0
Gravel	65.4
Sand	19.6
Silt	12.9
Clay	2.1

Grading Analysis		
D100	mm	
D60	mm	7.82
D30	mm	1.16
D10	mm	0.0169
Uniformity Coefficient		460
Curvature Coefficient		10

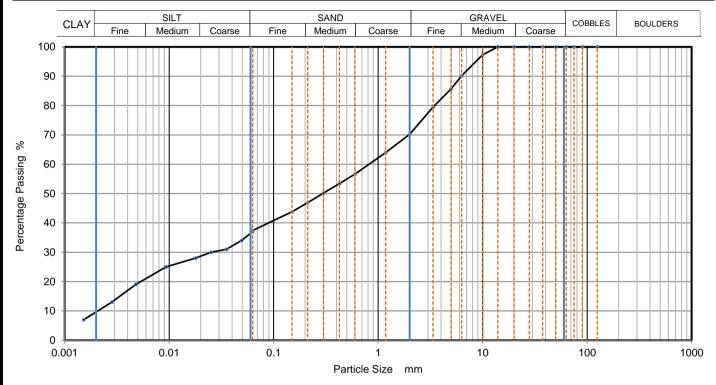
Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved

Stephen.Watson

CAUSEWAY	DART	PARTICLE SIZE DISTRIBUTION -		Job Ref	20-0399B	
GEOTECH	PARI			Borehole/Pit No.	R6-CP10	
Site Name	Bus Connects Route 6	Bus Connects Route 6 - Lucan to City Centre			Sample No.	4
Soil Description	Greyish brown sandy gravelly silty CLAY.			Depth, m	1.00	
Specimen Reference	9 Specimen 1 m			Sample Type	В	
Test Method	BS1377:Part 2:1990, clauses 9.2 and 9.5			KeyLAB ID	Caus2020103037	



Sieving		Sedimentation		
Particle Size mm	% Passing	Particle Size mm	% Passing	
125	100	0.06300	37	
90	100	0.04939	34	
75	100	0.03537	31	
63	100	0.02517	30	
50	100	0.01791	28	
37.5	100	0.00936	25	
28	100	0.00479	19	
20	100	0.00283	13	
14	100	0.00152	7	
10	97			
6.3	90			
5	86			
3.35	80			
2	70			
1.18	64			
0.6	57	Particle density	(assumed)	
0.425	53	2.65	Mg/m3	
0.3	50			
0.212	47			
0.15	44			
0.063	37			

Dry Mass of sample, g	508

Sample Proportions	% dry mass
Cobbles	0.0
Gravel	29.8
Sand	32.7
Silt	27.4
Clay	10.1

Grading Analysis		
D100	mm	
D60	mm	0.817
D30	mm	0.0256
D10	mm	0.00197
Uniformity Coefficient		410
Curvature Coefficient		0.41

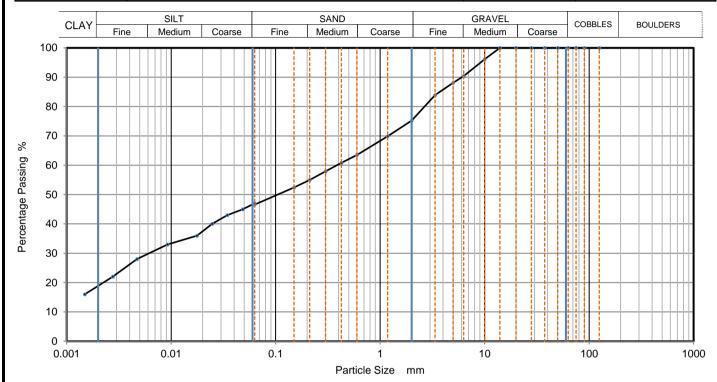
Preparation and testing in accordance with BS1377-2:1990 unless noted below



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Stephen.Watson

CAUSEWAY	DART	PARTICLE SIZE DISTRIBUTION -		Job Ref	20-0399B	
—— GEOTECH	PANI			Borehole/Pit No.	R6-CP11	
Site Name	Bus Connects Route 6	Bus Connects Route 6 - Lucan to City Centre			Sample No.	8
Soil Description	Greyish brown sandy slightly gravelly silty CLAY.			Depth, m	2.00	
Specimen Reference	9 Specimen 2 m Depth			Sample Type	В	
Test Method	BS1377:Part 2:1990, clauses 9.2 and 9.5			KeyLAB ID	Caus2020103041	



Sieving		Sedimentation		
Particle Size mm	% Passing	Particle Size mm	% Passing	
125	100	0.06300	47	
90	100	0.04843	45	
75	100	0.03447	43	
63	100	0.02470	40	
50	100	0.01769	36	
37.5	100	0.00925	33	
28	100	0.00471	28	
20	100	0.00277	22	
14	100	0.00149	16	
10	96			
6.3	90			
5	88			
3.35	84			
2	75			
1.18	70			
0.6	64	Particle density	(assumed)	
0.425	61	2.65	Mg/m3	
0.3	58			
0.212	55	1		
0.15	52	1		
0.063	47			

Sample Proportions	% dry mass
Cobbles	0.0
Gravel	24.8
Sand	28.7
Silt	27.7
Clay	18.8

Grading Analysis		
D100	mm	
D60	mm	0.385
D30	mm	0.00643
D10	mm	
Uniformity Coefficient		
Curvature Coefficient		

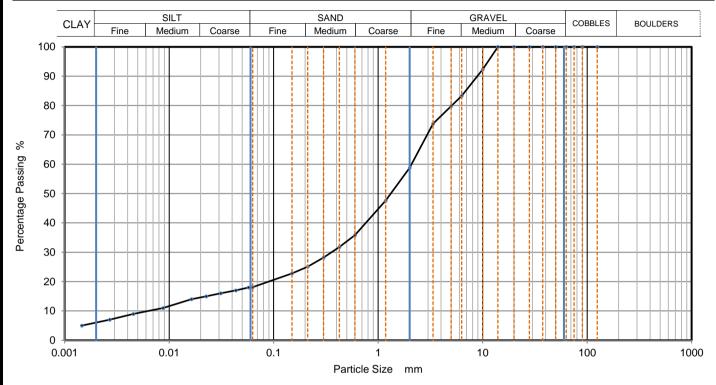
Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved

Stephen.Watson

CAUSEWAY	DART	ICLE SIZE DIST	Job Ref	20-0399B			
—— GEOTECH	PARI	ICLE SIZE DIST	IKIBUTIUN	Borehole/Pit No.	R6-WS01		
Site Name	Bus Connects Route 6	Bus Connects Route 6 - Lucan to City Centre				2	
Soil Description	Greyish brown sandy gravelly silty CLAY.				Depth, m	0.40	
Specimen Reference	3	Specimen Depth	0.4	Sample Type	В		
Test Method	BS1377:Part 2:1990, clauses 9.2 and 9.5				KeyLAB ID	Caus2020103043	



Siev	/ing	Sedimentation			
Particle Size mm	% Passing	Particle Size mm	% Passing		
125	100	0.05742	18		
90	100	0.04343	17		
75	100	0.03122	16		
63	100	0.02260	15		
50	100	0.01635	14		
37.5	100	0.00875	11		
28	100	0.00455	9		
20	100	0.00269	7		
14	100	0.00146	5		
10	92				
6.3	83				
5	80				
3.35	74				
2	59				
1.18	48				
0.6	36	Particle density	(assumed)		
0.425	32	2.65	Mg/m3		
0.3	28				
0.212 25					
0.15	23				
0.063	18				

Dry Mass of sample, g	511
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Sample Proportions	% dry mass
Cobbles	0.0
Gravel	41.2
Sand	40.7
Silt	12.1
Clay	6.0

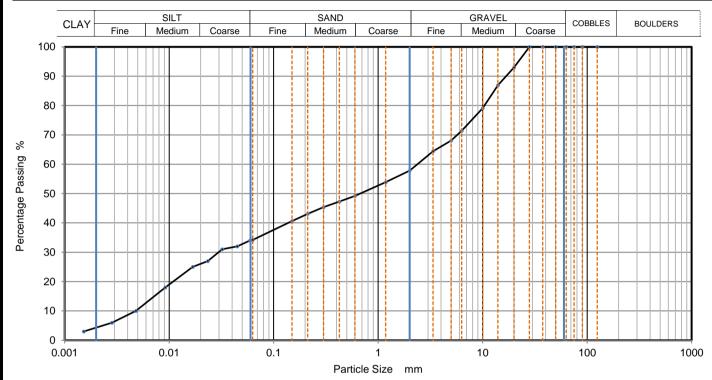
Grading Analysis		
D100	mm	
D60	mm	2.08
D30	mm	0.358
D10	mm	0.00607
Uniformity Coefficient		340
Curvature Coefficient		10

Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved
Stephen.Watson

CAUSEWAY	DART	ICLE SIZE DIST	FRIRITION	Job Ref	20-0399B	
——— GEOTECH	PARI	ICLE SIZE DIST	IKIBUTIUN	Borehole/Pit No.	R6-WS02	
Site Name	Bus Connects Route 6	Bus Connects Route 6 - Lucan to City Centre				1
Soil Description	Greyish brown sandy gravelly silty CLAY.				Depth, m	0.30
Specimen Reference	3	Specimen Depth	0.3	Sample Type	В	
Test Method	BS1377:Part 2:1990, clauses 9.2 and 9.5				KeyLAB ID	Caus2020103044



Siev	/ing	Sedimentation			
Particle Size mm	% Passing	Particle Size mm	% Passing		
125	100	0.05937	34		
90	100	0.04486	32		
75	100	0.03221	31		
63	100	0.02346	27		
50	100	0.01682	25		
37.5	100	0.00916	18		
28	100	0.00480	10		
20	93	0.00283	6		
14	87	0.00152	3		
10	79				
6.3	71				
5	68				
3.35	64				
2	58				
1.18	54				
0.6	49	Particle density	(assumed)		
0.425	47	2.65	Mg/m3		
0.3	45				
0.212	43				
0.15	41				
0.063	34				

Dry Mass of sample, g	2045
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Sample Proportions	% dry mass				
Cobbles	0.0				
Gravel	42.2				
Sand	23.6				
Silt	29.8				
Clay	4.4				

Grading Analysis		
D100	mm	
D60	mm	2.38
D30	mm	0.0309
D10	mm	0.00469
Uniformity Coefficient		510
Curvature Coefficient		0.085

Preparation and testing in accordance with BS1377-2:1990 unless noted below



Approved

Stephen.Watson



## **Moisture Condition Value at Natural Moisture Content Summary of Results**

Project No.

Project Name

20-0399B

Bus Connects Route 6 - Lucan to City Centre

	20 0000B Bus doffices Notice of Education Oily Octific									
Hole No.		Sar	mple		Soil Description	Retained on 20mm sieve	Moisture Content <20mm	Moisture Condition Value	Method of Interpretation	Remarks
Hole No.	Ref Top Base Type	Soil Description	%	%			Remarks			
R6-CP07	7	1.00		В	Brown sandy slightly gravelly silty CLAY.	30	19	9.3	Best fit line	
	•		•	-					L <i>P</i>	AB 10R Version 5

Key

Test performed in accordance with BS1377:Part4:1990, clause 5.4 unless

annotated otherwise

Date Printed

Approved By

18/11/2020

Stephen.Watson



CAUSEWAY	Unconsolidate Compression	Job Ref	20-0399B			
GEOTECH	of pore press			CIIC	Borehole/Pit No.	R6-CP09
Site Name	Bus Connects Route	e 6 - Lucan to City	Centre		Sample No.	15
Soil Description	Greyish brown sand	y silty CLAY.			Depth	3.00
Specimen Reference	6	Specimen Depth	3.05	m	Sample Type	U
Specimen Description	Very soft greyish bro	own sandy silty Cl	_AY.		KeyLAB ID	Caus2020103036
Test Method	BS1377 : Part 7 : 19	90, clause 8, sing	gle specimen		Date of test	09/11/2020

Test Number Length Diameter **Bulk Density** Moisture Content Dry Density

Rate of Strain Cell Pressure At failure

Axial Strain Deviator Stress, ( $\sigma$ 1 -  $\sigma$ 3)f Undrained Shear Strength, cu

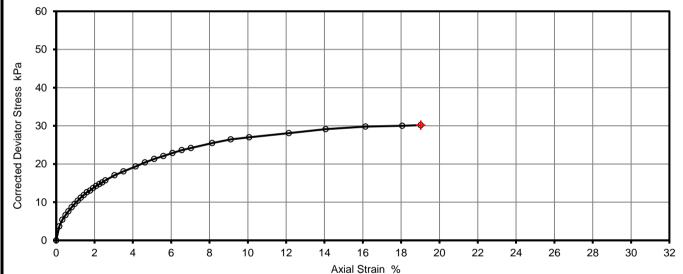
Mode of Failure

1	
208.2	mm
105.2	mm
2.10	Mg/m3
22.5	%
1.71	Mg/m3

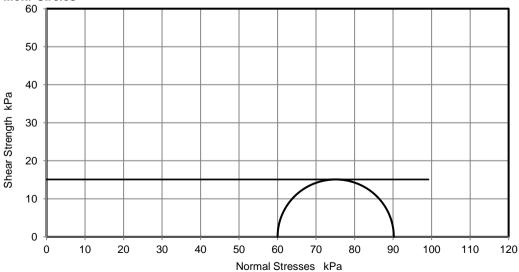
2.0	%/min
60	kPa
19.0	%
30	kPa
15	kPa ½

½( σ1 - σ3 )f

#### **Deviator Stress v Axial Strain**







Deviator stress corrected for area change and membrane effects based on Fig 11 BS1377-7:1990

Mohr circles and their interpretation is not covered by BS1377-7. This is provided for information only.

Remarks

No failure defined. Testing terminated at 20% axial strain.

Approved

Stephen.Watson

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19/11/2020 08:54

LAB 15R Version 4



· C	AUSEW GEOTE	AY			Point Load Strength Index Tests Summary of Results													
Project No. 20	)-0399B			Proje	ect Nam	е		Bus C	onnec	ts Ro	ute 6 - I	Lucan	to City	Centre				
Borehole	Sa	ample		Spe	ecimen	6.17	Test Type see ISRM		st Type (N/X)		Dimensions		nensions		Equivalent diameter, De	Point Strengt		Remarks (including
No.	Depth	Ref.	Туре	Ref.	Depth	Rock Type	Type (D, A, I, B)	Direction (L, P or U)	Failure Valid (Y/N)	Lne	W	Dps	Dps'		Equival	Is	Is(5 0)	water content if measured)
R6-CP07	m 6.85		С	1	m 6.85	LIMESTONE	A	U	NO	mm	mm 101.5	mm 56.0	mm 55.0	kN 23.2	mm 84.3	MPa 3.3	MPa 4.1	
						LIMESTONE												
R6-CP07	7.60		С	2	7.60	LIMESTONE	D	U	NO	68.3	101.6		99.0	30.1	100.3		4.1	
R6-CP07	7.80		С	3	7.80	LIMESTONE	D	U	YES	58.5	101.6	101.6	98.0	11.6	99.8	1.2	1.6	
R6-CP07	8.50		С	4	8.50	LIMESTONE	D	U	YES	71.0	101.7	101.7	98.0	10.1	99.8	1.0	1.4	
R6-CP07	9.40		С	5	9.40	LIMESTONE	I	U	NO	102.1	80.7	58.0	56.0	5.6	75.9	1.0	1.2	
R6-CP07	10.60		С	6	10.60	LIMESTONE	А	U	NO	101.6	101.6	62.0	60.0	5.2	88.1	0.7	0.9	
Test Type					5.			Avio				Plo	ole.					
Direction L - parallel to plane P - perpendicular t U - unknown or rai Dimensions Dps - Distance bet Dps' - at failure ( s Lne - Length from	es of weakn to planes of indom tween plate ee ISRM no platens to r	of weakness  Dps  Dps  Une  Dps  Dps  Dps  Dps  Dps  Dps  Dps  Dp																
Test performed in Detailed legend fo							noted o	therwis	se		Date F	Printed 7/11/20	20	Appro	ved B	<b>_</b>	lumlum	
Size factor, F = (D	0e/50)0.45	for all t	ests.			1	LAB 1	7R V	'ersio	า 4				Stenk	nen W	/atson		U K A S TESTING 10122



## **UNIAXIAL COMPRESSION TEST ON ROCK - SUMMARY OF RESULTS**

Project No.

Project Name

20-0399B

Bus Connects Route 6 - Lucan to City Centre

		Sar	mple			S Dir	Specime mensior	n ns2	Bulk	Water		ial Compre	ession3	
Hole No.	Ref	Тор	Base	Туре	Rock Type	Dia.	Length	H/D	Density2	Content 1	Condition	Mode of failure	UCS	Remarks
						mm	mm		Mg/m3	%		laliule	MPa	
R6-CP07		8.85	9.05	O	LIMESTONE	101.6	254.0	2.5	2.67	0.6	as received	F	35.6	
					105 ± 3 oC, specimen a			and deriv	ation of bulk	density	Mode of failu S - Single sho		MS - multiple	e shear

3 ISRM p153 part 1, determination of Uniaxial Compressive Strength ( UCS ) of Rock Materials

above notes apply unless annotated otherwise in the remarks

AC - Axial cleavage

F - Fragmented

Test Specification	Date Printed	Approved By	Table	
International Society for Rock Mechanics, The complete ISRM suggested methods for Rock Characterization Testing and Monitoring, 2007	17/11/2020			1
			sheet	
		Stephen.Watson		1



eurofins

Chemtest
Eurofins Chemtest Ltd
Depot Road
Newmarket
CB8 0AL

Tel: 01638 606070 Email: info@chemtest.com

# **Final Report**

**Report No.:** 20-30010-1

Initial Date of Issue: 10-Nov-2020

Client Causeway Geotech Ltd

Client Address: 8 Drumahiskey Road

Balnamore Ballymoney County Antrim BT53 7QL

Contact(s): Carin Cornwall

Colm Hurley
Darren O'Mahony
Gabriella Horan
Joe Gervin
John Cameron
Lucy Newland
Martin Gardiner
Matthew Gilbert
Neil Haggan
Paul Dunlop
Sean Ross
Stephen Franey

Stephen McCracken Stephen Watson Stuart Abraham Thomas McAllis

**Project** 20-0399B Bus Connects Route 6

Quotation No.: Date Received: 05-Nov-2020

Order No.: Date Instructed: 05-Nov-2020

No. of Samples: 16

Turnaround (Wkdays): 5 Results Due: 11-Nov-2020

Date Approved: 10-Nov-2020

Approved By:

**Details:** Glynn Harvey, Technical Manager



Eurofins Chemtest Ltd Depot Road . Newmarket CB8 0AL

Tel: 01638 606070

Email: info@chemtest.com

Client: Causeway Geotech Ltd		Cher	mtest J	ob No.:	20-30010	20-30010	20-30010	20-30010	20-30010	20-30010	20-30010	20-30010	20-30010
Quotation No.:	(	Chemtest Sample ID.:		1092206	1092207	1092208	1092209	1092210	1092211	1092212	1092213	1092214	
Order No.:		Client Sample Ref.:		10	1	5	6	6	7	12		7	
		Clie	ent Sam	ple ID.:	R6-CP03	R6-CP04	R6-CP04	R6-CP05	R6-CP06	R6-CP07	R6-CP07	R6-CP07	R6-CP08
			Sampl	е Туре:	SOIL								
			Top De <sub>l</sub>	oth (m):	2.00	0.10	1.70	2.50	1.90	1.00	3.20	5.20	2.00
			Date Sa	ampled:	04-Nov-2020								
Determinand	Accred.	SOP	Units	LOD									
Moisture	N	2030	%	0.020	14	9.2	11	7.7	8.6	14	20	12	13
рН	U	2010		4.0	8.8	8.7	8.5	8.7	8.9	8.6	8.2	8.8	8.9
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	< 0.010	0.069	0.024	< 0.010

Client: Causeway Geotech Ltd		Cher	ntest Jo	ob No.:	20-30010	20-30010	20-30010	20-30010	20-30010	20-30010	20-30010
Quotation No.:	(	Chemte	st Sam	ple ID.:	1092215	1092216	1092217	1092218	1092219	1092220	1092221
Order No.:		Client Sample Ref.:		11	8	3	3	11	2	1	
		Clie	ent Sam	ple ID.:	R6-CP09	R6-CP09	R6-CP10	R6-CP11	R6-CP11	R6-WS01	R6-WS02
			Sample	е Туре:	SOIL						
			Top Dep	oth (m):	1.20	3.00	1.20	1.20	3.00	0.40	0.30
			Date Sa	ampled:	04-Nov-2020						
Determinand	Accred.	SOP	Units	LOD							
Moisture	N	2030	%	0.020	17	15	7.3	8.1	11	7.2	8.1
рН	U	2010		4.0	8.6	8.6	8.5	8.2	9.4	8.7	8.8
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	0.025	< 0.010	0.082	1.2	0.21	< 0.010	< 0.010

# **Test Methods**

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2040	Soil Description(Requirement of MCERTS)	Soil description	As received soil is described based upon BS5930
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES

### **Report Information**

# Key

- U UKAS accredited
- M MCERTS and UKAS accredited
- N Unaccredited
- S This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
- SN This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
- T This analysis has been subcontracted to an unaccredited laboratory
- I/S Insufficient Sample
- U/S Unsuitable Sample
- N/E not evaluated
  - < "less than"
  - > "greater than"

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

#### **Sample Deviation Codes**

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

#### **Sample Retention and Disposal**

All soil samples will be retained for a period of 45 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>



# eurofins Chemtest

Eurofins Chemtest Ltd
Depot Road
Newmarket
CB8 0AL

Tel: 01638 606070 Email: info@chemtest.com

# **Final Report**

**Report No.:** 20-31075-1

Initial Date of Issue: 19-Nov-2020

Client Causeway Geotech Ltd

Client Address: 8 Drumahiskey Road

Balnamore Ballymoney County Antrim BT53 7QL

Contact(s): Carin Cornwall

Colm Hurley
Darren O'Mahony
Gabriella Horan
Joe Gervin
John Cameron
Lucy Newland
Martin Gardiner
Matthew Gilbert
Neil Haggan
Paul Dunlop
Sean Ross
Stephen Franey

Stephen McCracken Stephen Watson Stuart Abraham Thomas McAllis

Project 20-0399B Route 6 Lucan to City Centre

Quotation No.: Date Received: 16-Nov-2020

Order No.: Date Instructed: 16-Nov-2020

No. of Samples: 1

Turnaround (Wkdays): 5 Results Due: 20-Nov-2020

Date Approved: 19-Nov-2020

Approved By:

Details: Glynn Harvey, Technical Manager



Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL

Tel: 01638 606070

Email: info@chemtest.com

# Project: 20-0399B Route 6 Lucan to City Centre

Client: Causeway Geotech Ltd		Che	ntest Jo	ob No.:	20-31075
Quotation No.:	(	Chemtest Sample ID.:			
	Sample Location:				R6-CP07
	Sample Type:			SOIL	
	Top Depth (m):				6.85
			Date Sa	ampled:	13-Nov-2020
Determinand	Accred.	SOP	Units	LOD	
Moisture	N	2030	%	0.020	1.9
рН	U	2010		4.0	8.6
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	0.088

# **Test Methods**

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2040	Soil Description(Requirement of MCERTS)	Soil description	As received soil is described based upon BS5930
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES

### **Report Information**

# Key

- U UKAS accredited
- M MCERTS and UKAS accredited
- N Unaccredited
- S This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
- SN This analysis has been subcontracted to a UKAS accredited laboratory that is not accredited for this analysis
- T This analysis has been subcontracted to an unaccredited laboratory
- I/S Insufficient Sample
- U/S Unsuitable Sample
- N/E not evaluated
  - < "less than"
  - > "greater than"

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

#### **Sample Deviation Codes**

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

#### **Sample Retention and Disposal**

All soil samples will be retained for a period of 45 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>



## LABORATORY RESTRICTION REPORT

Project Reference	20-0399B			То	Sean Ross
Project Name	Bus Connects Route 9 - Lucan to City	Cent	re	Position	Project Manager
- reject riame				From	Joseph Nicholl
TR reference	20-0399B	/	G01	Position	Laboratory Quality Manager

The following sample(s) and test(s) are restricted as detailed below. Could you please complete the "Required Action" column and return the completed form to the laboratory.

Hole			Test		5 4 5 4 4	Described Astion		
Number	Number	Depth (m)	Туре	Type	Reason for Restriction	Required Action		
R6 CP01	14	3.00	U	UU Triaxial	Unable to obtain specimen for test - coarse gravel content too high	CANCEL		
R6 CP03	12	1.20	U	UU Triaxial	Unable to obtain specimen for test - sample broken by layer of sand	CANCEL		
R6 CP04	6	2.00	U	UU Triaxial	Unable to obtain specimen for test - sample broken on extrusion	CANCEL		
R6 CP05	5	2.00	U	UU Triaxial	Unable to obtain specimen for test - coarse gravel content too high	CANCEL		
R6 CP06	9	2.00	U	UU Triaxial	Unable to obtain specimen for test - cobbles present in sample	CANCEL		
R6 CP07	7	1.00	В	UU Triaxial	Unable to obtain specimen for test - cobbles present in sample	CANCEL		
R6 CP07		6.20	С	UU Triaxial	Unable to obtain specimen for test - cobbles present in sample	CANCEL		
R6 CP08	13	2.00	U	UU Triaxial	Unable to obtain specimen for test - coarse gravel content too high	CANCEL		
For electronic reporting a form of electronic signature or printed name is				e is	Laboratory Signature Joseph Nicholl	Project Manager Signature Sean Ross		
acceptab	le				Date 10 November 2020	Date		



# APPENDIX G ENVIRONMENTAL LABORATORY TEST RESULTS





# eurofins Chemtest

Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL

Tel: 01638 606070 Email: info@chemtest.com

# **Final Report**

**Report No.:** 20-26018-1

Initial Date of Issue: 02-Oct-2020

Client Causeway Geotech Ltd

Client Address: 8 Drumahiskey Road

Balnamore Ballymoney County Antrim BT53 7QL

Contact(s): Carin Cornwall

Colm Hurley
Darren O'Mahony
Gabriella Horan
Joe Gervin
John Cameron
Lucy Newland
Martin Gardiner
Matthew Gilbert
Neil Haggan
Paul Dunlop
Sean Ross
Stephen Franey

Stephen McCracken Stephen Watson Stuart Abraham Thomas McAllis

**Project** 20-0399B Bus Connects Route 6

Quotation No.: Q20-21063 Date Received: 28-Sep-2020

Order No.: Date Instructed: 28-Sep-2020

No. of Samples: 1

Turnaround (Wkdays): 5 Results Due: 02-Oct-2020

Date Approved: 02-Oct-2020

Approved By:

**Details:** Glynn Harvey, Technical Manager



Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL

Tel: 01638 606070 Email: info@chemtest.com

# Results - Leachate

Client: Causeway Geotech Ltd			Che	mtest Jo	ob No.:	20-26018
Quotation No.: Q20-21063		Chemtest Sample ID.:				
		Sample Location:				R6-CP07
	Sample Type:				SOIL	
	Top Depth (m):				0.50	
				Date Sa	ampled:	25-Sep-2020
Determinand	Accred. SOP Type Units LOD					
Ammonium	U	1220	10:1	mg/l	0.050	0.45
Ammonium	N	1220	10:1	mg/kg	0.10	9.3

Client: Causeway Geotech Ltd		Chemtest Job No.:				
Quotation No.: Q20-21063	(		st Sam		1071187	
		Sa	ample Lo		R6-CP07 SOIL	
		Sample Type:				
		Top Depth (m):				
			Date Sa	ampled:	25-Sep-2020	
			Asbest	os Lab:	COVENTRY	
Determinand	Accred.	SOP	Units	LOD		
ACM Type	U	2192		N/A	=	
Asbestos Identification	U	2192		N/A	No Asbestos Detected	
ACM Detection Stage	U	2192		N/A	-	
Moisture	N	2030	%	0.020	14	
pН	М	2010		4.0	8.7	
Boron (Hot Water Soluble)	М		mg/kg		0.56	
Sulphur (Elemental)	М		mg/kg	1.0	3.2	
Cyanide (Total)	М		mg/kg	0.50	< 0.50	
Sulphide (Easily Liberatable)	N	2325		0.50	9.1	
Sulphate (Total)	М	2430	%	0.010	0.023	
Arsenic	М	2450	mg/kg	1.0	4.8	
Barium	М	2450		10	42	
Cadmium	М		mg/kg		< 0.10	
Chromium	М		mg/kg	1.0	29	
Molybdenum	М		mg/kg	2.0	< 2.0	
Antimony	N		mg/kg		< 2.0	
Copper	М	2450	mg/kg	0.50	15	
Mercury	М	2450		0.10	< 0.10	
Nickel	М	2450	J	0.50	25	
Lead	М	2450	mg/kg		14	
Selenium	М		mg/kg	0.20	< 0.20	
Zinc	М		mg/kg	0.50	50	
Chromium (Trivalent)	N	2490		1.0	29	
Chromium (Hexavalent)	N	2490		0.50	< 0.50	
Total Organic Carbon	М	2625	%	0.20	1.4	
Mineral Oil	N	2670		10	< 10	
Aliphatic TPH >C5-C6	N	2680		1.0	< 1.0	
Aliphatic TPH >C6-C8	N		mg/kg	1.0	< 1.0	
Aliphatic TPH >C8-C10	М		mg/kg	1.0	< 1.0	
Aliphatic TPH >C10-C12	М	2680		1.0	< 1.0	
Aliphatic TPH >C12-C16	М	2680	mg/kg	1.0	< 1.0	
Aliphatic TPH >C16-C21	М	2680		1.0	< 1.0	
Aliphatic TPH >C21-C35	М	2680	mg/kg	1.0	< 1.0	
Aliphatic TPH >C35-C44	N		mg/kg	1.0	< 1.0	
Total Aliphatic Hydrocarbons	N		mg/kg	5.0	< 5.0	
Aromatic TPH >C5-C7	N		mg/kg	1.0	< 1.0	
Aromatic TPH >C7-C8	N	2680		1.0	< 1.0	
Aromatic TPH >C8-C10	М		mg/kg	1.0	< 1.0	

Client: Causeway Geotech Ltd		Chemtest Job No.:				
Quotation No.: Q20-21063		Chemtest Sample ID.				
		Sa	ample Lo		R6-CP07 SOIL	
		Sample Type				
		Top Depth (m):				
			Date Sa	ampled:	25-Sep-2020	
			Asbest	os Lab:	COVENTRY	
Determinand	Accred.	SOP	Units	LOD		
Aromatic TPH >C10-C12	М	2680	mg/kg	1.0	< 1.0	
Aromatic TPH >C12-C16	M	2680	mg/kg	1.0	< 1.0	
Aromatic TPH >C16-C21	U	2680	mg/kg	1.0	< 1.0	
Aromatic TPH >C21-C35	М	2680	mg/kg	1.0	< 1.0	
Aromatic TPH >C35-C44	N	2680	mg/kg	1.0	< 1.0	
Total Aromatic Hydrocarbons	N	2680		5.0	< 5.0	
Total Petroleum Hydrocarbons	N		mg/kg		< 10	
Benzene	М	2760	J	1.0	< 1.0	
Toluene	М	2760		1.0	< 1.0	
Ethylbenzene	М	2760		1.0	< 1.0	
m & p-Xylene	М	2760	μg/kg	1.0	< 1.0	
o-Xylene	М	2760		1.0	< 1.0	
Methyl Tert-Butyl Ether	M	2760		1.0	< 1.0	
Naphthalene	M		mg/kg		< 0.10	
Acenaphthylene	N		mg/kg		< 0.10	
Acenaphthene	M		mg/kg		< 0.10	
Fluorene	M		mg/kg		< 0.10	
Phenanthrene	M	2800			< 0.10	
Anthracene	M	2800			< 0.10	
Fluoranthene	M	2800			< 0.10	
Pyrene	M		mg/kg		< 0.10	
Benzo[a]anthracene	M		mg/kg		< 0.10	
Chrysene	M		mg/kg		< 0.10	
Benzo[b]fluoranthene	M	2800			< 0.10	
Benzo[k]fluoranthene	M		mg/kg		< 0.10	
Benzo[a]pyrene	M		mg/kg		< 0.10	
Indeno(1,2,3-c,d)Pyrene	M	2800			< 0.10	
Dibenz(a,h)Anthracene	N	2800			< 0.10	
Benzo[g,h,i]perylene	M		mg/kg	0.10	< 0.10	
Coronene	N N		mg/kg		< 0.10	
Total Of 17 PAH's	N N	2800			< 2.0	
PCB 28	U		mg/kg		< 0.010	
PCB 52	U	2815			< 0.010	
PCB 90+101	U		mg/kg		< 0.010	
PCB 118	U		mg/kg		< 0.010	
PCB 118						
PCB 138	U		mg/kg		< 0.010	
			mg/kg		< 0.010	
PCB 180	U		mg/kg		< 0.010	
Total PCBs (7 Congeners)	U	2815	mg/kg	0.10	< 0.10	

Client: Causeway Geotech Ltd		Che	mtest Jo	b No.:	20-26018	
Quotation No.: Q20-21063	(	Chemtest Sample ID.:				
		Sample Location:				
		Sample Type:				
	Top Depth (m):				0.50	
		Date Sampled:				
			Asbest	os Lab:	COVENTRY	
Determinand	Accred.	SOP	Units	LOD		
Total Phenols	М	2920	mg/kg	0.30	< 0.30	

# **Results - Single Stage WAC**

Project: 20-0399B Bus Connects Route 6

Project: 20-0399B Bus Connects I					1leur v	N1- A	- Oultania
Chemtest Job No:	20-26018				Landilli	Waste Acceptanc	e Criteria
Chemtest Sample ID:	1071187					Limits	1
Sample Ref:						Stable, Non-	
Sample ID:	Do 0007					reactive	
Sample Location:	R6-CP07					hazardous	Hazardous
Top Depth(m):	0.50				Inert Waste	waste in non-	Waste
Bottom Depth(m):					Landfill	hazardous	Landfill
Sampling Date:	25-Sep-2020					Landfill	
Determinand	SOP	Accred.	Units				
Total Organic Carbon	2625	M	%	1.4	3	5	6
Loss On Ignition	2610	M	%	8.7			10
Total BTEX	2760	M	mg/kg	< 0.010	6		
Total PCBs (7 Congeners)	2815	M	mg/kg	< 0.10	1		
TPH Total WAC (Mineral Oil)	2670	M	mg/kg	< 10	500		
Total (Of 17) PAH's	2800	N	mg/kg	< 2.0	100		
рН	2010	М		8.7		>6	
Acid Neutralisation Capacity	2015	N	mol/kg	0.057		To evaluate	To evaluate
Eluate Analysis			10:1 Eluate	10:1 Eluate	Limit values	for compliance l	eaching test
			mg/l	mg/kg	using B	S EN 12457 at L/	S 10 l/kg
Arsenic	1450	U	< 0.0010	< 0.050	0.5	2	25
Barium	1450	U	0.0077	< 0.50	20	100	300
Cadmium	1450	U	< 0.00010	< 0.010	0.04	1	5
Chromium	1450	U	< 0.0010	< 0.050	0.5	10	70
Copper	1450	U	0.0070	0.070	2	50	100
Mercury	1450	U	< 0.00050	< 0.0050	0.01	0.2	2
Molybdenum	1450	U	0.0031	< 0.050	0.5	10	30
Nickel	1450	U	0.0030	< 0.050	0.4	10	40
Lead	1450	U	< 0.0010	< 0.010	0.5	10	50
Antimony	1450	U	< 0.0010	< 0.010	0.06	0.7	5
Selenium	1450	U	< 0.0010	< 0.010	0.1	0.5	7
Zinc	1450	U	0.0091	< 0.50	4	50	200
Chloride	1220	U	12	120	800	15000	25000
Fluoride	1220	U	0.35	3.5	10	150	500
Sulphate	1220	U	12	120	1000	20000	50000
Total Dissolved Solids	1020	N	140	1400	4000	60000	100000
Phenol Index	1920	U	< 0.030	< 0.30	1	-	-
Dissolved Organic Carbon	1610	U	4.8	< 50	500	800	1000

Solid Information	
Dry mass of test portion/kg	0.090
Moisture (%)	14

# **Waste Acceptance Criteria**

Landfill WAC analysis (specifically leaching test results) must not be used for hazardous waste classification purposes. This analysis is only applicable for hazardous waste landfill acceptance and does not give any indication as to whether a waste may be hazardous or non-hazardous.

# **Test Methods**

SOP	Title	Parameters included	Method summary
1010	pH Value of Waters	pH	pH Meter
1020	Electrical Conductivity and	Electrical Conductivity and Total Dissolved Solids (TDS) in Waters	Conductivity Meter
1220	Anions, Alkalinity & Ammonium in Waters	Fluoride; Chloride; Nitrite; Nitrate; Total; Oxidisable Nitrogen (TON); Sulfate; Phosphate; Alkalinity; Ammonium	Automated colorimetric analysis using 'Aquakem 600' Discrete Analyser.
1450	Metals in Waters by ICP-MS	Metals, including: Antimony; Arsenic; Barium; Beryllium; Boron; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Tin; Vanadium; Zinc	Filtration of samples followed by direct determination by inductively coupled plasma mass spectrometry (ICP-MS).
1610	Total/Dissolved Organic Carbon in Waters	Organic Carbon	TOC Analyser using Catalytic Oxidation
1920	Phenols in Waters by HPLC	Phenolic compounds including: Phenol, Cresols, Xylenols, Trimethylphenols Note: Chlorophenols are excluded.	Determination by High Performance Liquid Chromatography (HPLC) using electrochemical detection.
2010	pH Value of Soils	pH	pH Meter
2015	Acid Neutralisation Capacity	Acid Reserve	Titration
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2040	Soil Description(Requirement of MCERTS)	Soil description	As received soil is described based upon BS5930
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2180	Sulphur (Elemental) in Soils by HPLC	Sulphur	Dichloromethane extraction / HPLC with UV detection
2192	Asbestos	Asbestos	Polarised light microscopy / Gravimetry
2300	Cyanides & Thiocyanate in Soils	Free (or easy liberatable) Cyanide; total Cyanide; complex Cyanide; Thiocyanate	Allkaline extraction followed by colorimetric determination using Automated Flow Injection Analyser.
2325	Sulphide in Soils	Sulphide	Steam distillation with sulphuric acid / analysis by 'Aquakem 600' Discrete Analyser, using N,N–dimethyl-p-phenylenediamine.
2430	Total Sulphate in soils	Total Sulphate	Acid digestion followed by determination of sulphate in extract by ICP-OES.
2450	Acid Soluble Metals in Soils	Metals, including: Arsenic; Barium; Beryllium; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Vanadium; Zinc	Acid digestion followed by determination of metals in extract by ICP-MS.
2490	Hexavalent Chromium in Soils	Chromium [VI]	Soil extracts are prepared by extracting dried and ground soil samples into boiling water. Chromium [VI] is determined by 'Aquakem 600' Discrete Analyser using 1,5-diphenylcarbazide.
2610	Loss on Ignition	loss on ignition (LOI)	Determination of the proportion by mass that is lost from a soil by ignition at 550°C.
2625	Total Organic Carbon in Soils	Total organic Carbon (TOC)	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2670	Total Petroleum Hydrocarbons (TPH) in Soils by GC-FID	TPH (C6–C40); optional carbon banding, e.g. 3- band – GRO, DRO & LRO*TPH C8–C40	Dichloromethane extraction / GC-FID
2680	TPH A/A Split	Aliphatics: >C5-C6, >C6-C8,>C8-C10, >C10-C12, >C12-C16, >C16-C21, >C21- C35, >C35- C44Aromatics: >C5-C7, >C7-C8, >C8-C10, >C10-C12, >C12-C16, >C16-C21, >C21-C35, >C35-C44	Dichloromethane extraction / GCxGC FID detection

# **Test Methods**

SOP	Title	Parameters included	Method summary
2760	Volatile Organic Compounds (VOCs) in Soils by Headspace GC-MS	Volatile organic compounds, including BTEX and halogenated Aliphatic/Aromatics.(cf. USEPA Method 8260)*please refer to UKAS schedule	Automated headspace gas chromatographic (GC) analysis of a soil sample, as received, with mass spectrometric (MS) detection of volatile organic compounds.
2800	Speciated Polynuclear Aromatic Hydrocarbons (PAH) in Soil by GC-MS	Acenaphthene*; Acenaphthylene; Anthracene*; Benzo[a]Anthracene*; Benzo[a]Pyrene*; Benzo[b]Fluoranthene*; Benzo[ghi]Perylene*; Benzo[k]Fluoranthene; Chrysene*; Dibenz[ah]Anthracene; Fluoranthene*; Fluorene*; Indeno[123cd]Pyrene*; Naphthalene*; Phenanthrene*; Pyrene*	Dichloromethane extraction / GC-MS
2815	Polychlorinated Biphenyls (PCB) ICES7Congeners in Soils by GC-MS	ICES7 PCB congeners	Acetone/Hexane extraction / GC-MS
2920	Phenols in Soils by HPLC	Phenolic compounds including Resorcinol, Phenol, Methylphenols, Dimethylphenols, 1- Naphthol and TrimethylphenolsNote: chlorophenols are excluded.	60:40 methanol/water mixture extraction, followed by HPLC determination using electrochemical detection.
640	Characterisation of Waste (Leaching C10)	Waste material including soil, sludges and granular waste	ComplianceTest for Leaching of Granular Waste Material and Sludge

### **Report Information**

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- T This analysis has been subcontracted to an unaccredited laboratory
- I/S Insufficient Sample
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- < "less than"
- > "greater than"

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The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

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All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

#### **Sample Deviation Codes**

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

#### **Sample Retention and Disposal**

All soil samples will be retained for a period of 45 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>



# eurofins Chemtest

Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL

Tel: 01638 606070 Email: info@chemtest.com

# **Final Report**

**Report No.:** 20-29269-1

Initial Date of Issue: 03-Nov-2020

Client Causeway Geotech Ltd

Client Address: 8 Drumahiskey Road

Balnamore Ballymoney County Antrim BT53 7QL

Contact(s): Carin Cornwall

Colm Hurley
Darren O'Mahony
Gabriella Horan
Joe Gervin
John Cameron
Lucy Newland
Martin Gardiner
Matthew Gilbert
Neil Haggan
Paul Dunlop
Sean Ross
Stephen Franey

Stephen Franey
Stephen McCracken
Stephen Watson
Stuart Abraham
Thomas McAllis

**Project** 20-0399B Bus Connects Route 6

Quotation No.: Q20-21063 Date Received: 29-Oct-2020

Order No.: Date Instructed: 29-Oct-2020

No. of Samples: 1

Turnaround (Wkdays): 5 Results Due: 04-Nov-2020

Date Approved: 03-Nov-2020

Approved By:

**Details:** Glynn Harvey, Technical Manager



Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL

Tel: 01638 606070

Email: info@chemtest.com

Client: Causeway Geotech Ltd		20-29269			
Quotation No.: Q20-21063		Chemte	st Sam	ple ID.:	1088344
		Sa	ample Lo	cation:	R6-CP09
		е Туре:	SOIL		
			Top Dep	oth (m):	1.00
			Date Sa	impled:	23-Oct-2020
			Asbest	os Lab:	COVENTRY
Determinand	Accred.	SOP	Units	LOD	
ACM Type	U	2192		N/A	-
Asbestos Identification	U	2192		N/A	No Asbestos Detected
ACM Detection Stage	U	2192		N/A	-
Moisture	N	2030	%	0.020	9.6
pH	U	2010		4.0	8.6
Boron (Hot Water Soluble)	U	2120	mg/kg	0.40	< 0.40
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	< 0.010
Cyanide (Total)	U	2300	mg/kg	0.50	< 0.50
Arsenic	U	2450	mg/kg	1.0	20
Cadmium	U	2450	mg/kg	0.10	1.4
Chromium	U	2450	mg/kg	1.0	13
Copper	U	2450	mg/kg	0.50	23
Mercury	U	2450	mg/kg	0.10	< 0.10
Nickel	U	2450	mg/kg	0.50	32
Lead	U	2450	mg/kg	0.50	27
Zinc	U	2450	mg/kg	0.50	56
Organic Matter	U	2625	%	0.40	2.2
Total TPH >C6-C40	U	2670	mg/kg	10	< 10
Naphthalene	U	2700	mg/kg	0.10	< 0.10
Acenaphthylene	U	2700	mg/kg	0.10	< 0.10
Acenaphthene	U	2700	mg/kg	0.10	< 0.10
Fluorene	U	2700	mg/kg	0.10	< 0.10
Phenanthrene	U	2700	mg/kg	0.10	0.66
Anthracene	U	2700	mg/kg	0.10	0.19
Fluoranthene	U	2700	mg/kg	0.10	1.3
Pyrene	U	2700	)	0.10	1.4
Benzo[a]anthracene	U	2700	mg/kg	0.10	0.72
Chrysene	U	2700	0	0.10	0.72
Benzo[b]fluoranthene	U	2700	mg/kg	0.10	< 0.10
Benzo[k]fluoranthene	U	2700	mg/kg	0.10	< 0.10
Benzo[a]pyrene	U	2700	,	0.10	< 0.10
Indeno(1,2,3-c,d)Pyrene	U	2700	)	0.10	< 0.10
Dibenz(a,h)Anthracene	U	2700		0.10	< 0.10
Benzo[g,h,i]perylene	U	2700	mg/kg	0.10	< 0.10
Coronene	N	2700	mg/kg	0.10	< 0.10
Total Of 17 PAH's	N	2700	)	2.0	5.0
Total Phenols	U	2920	mg/kg	0.30	< 0.30

# **Test Methods**

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2040	Soil Description(Requirement of MCERTS)	Soil description	As received soil is described based upon BS5930
2120	Water Soluble Boron, Sulphate, Magnesium & Chromium	Boron; Sulphate; Magnesium; Chromium	Aqueous extraction / ICP-OES
2192	Asbestos	Asbestos	Polarised light microscopy / Gravimetry
2300	Cyanides & Thiocyanate in Soils	Free (or easy liberatable) Cyanide; total Cyanide; complex Cyanide; Thiocyanate	Allkaline extraction followed by colorimetric determination using Automated Flow Injection Analyser.
2450	Acid Soluble Metals in Soils	Metals, including: Arsenic; Barium; Beryllium; Cadmium; Chromium; Cobalt; Copper; Lead; Manganese; Mercury; Molybdenum; Nickel; Selenium; Vanadium; Zinc	Acid digestion followed by determination of metals in extract by ICP-MS.
2625	Total Organic Carbon in Soils	Total organic Carbon (TOC)	Determined by high temperature combustion under oxygen, using an Eltra elemental analyser.
2670	Total Petroleum Hydrocarbons (TPH) in Soils by GC-FID	TPH (C6–C40); optional carbon banding, e.g. 3-band – GRO, DRO & LRO*TPH C8–C40	Dichloromethane extraction / GC-FID
2700	Speciated Polynuclear Aromatic Hydrocarbons (PAH) in Soil by GC-FID	Acenaphthene; Acenaphthylene; Anthracene; Benzo[a]Anthracene; Benzo[a]Pyrene; Benzo[b]Fluoranthene; Benzo[ghi]Perylene; Benzo[k]Fluoranthene; Chrysene; Dibenz[ah]Anthracene; Fluoranthene; Fluorene; Indeno[123cd]Pyrene; Naphthalene; Phenanthrene; Pyrene	Dichloromethane extraction / GC-FID (GC-FID detection is non-selective and can be subject to interference from co-eluting compounds)
2920	Phenols in Soils by HPLC	Phenolic compounds including Resorcinol, Phenol, Methylphenols, Dimethylphenols, 1- Naphthol and TrimethylphenolsNote: chlorophenols are excluded.	60:40 methanol/water mixture extraction, followed by HPLC determination using electrochemical detection.

### **Report Information**

# Key

- U UKAS accredited
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- S This analysis has been subcontracted to a UKAS accredited laboratory that is accredited for this analysis
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- I/S Insufficient Sample
- U/S Unsuitable Sample
- N/E not evaluated
  - < "less than"
  - > "greater than"

Comments or interpretations are beyond the scope of UKAS accreditation

The results relate only to the items tested

Uncertainty of measurement for the determinands tested are available upon request

None of the results in this report have been recovery corrected

All results are expressed on a dry weight basis

The following tests were analysed on samples as received and the results subsequently corrected to a dry weight basis TPH, BTEX, VOCs, SVOCs, PCBs, Phenols

For all other tests the samples were dried at < 37°C prior to analysis

All Asbestos testing is performed at the indicated laboratory

Issue numbers are sequential starting with 1 all subsequent reports are incremented by 1

#### **Sample Deviation Codes**

- A Date of sampling not supplied
- B Sample age exceeds stability time (sampling to extraction)
- C Sample not received in appropriate containers
- D Broken Container
- E Insufficient Sample (Applies to LOI in Trommel Fines Only)

#### **Sample Retention and Disposal**

All soil samples will be retained for a period of 45 days from the date of receipt

All water samples will be retained for 14 days from the date of receipt

Charges may apply to extended sample storage

If you require extended retention of samples, please email your requirements to: <u>customerservices@chemtest.com</u>



eurofins

Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL

Chemtest

Tel: 01638 606070 Email: info@chemtest.com

# **Final Report**

**Report No.:** 20-29273-1

Initial Date of Issue: 04-Nov-2020

Client Causeway Geotech Ltd

Client Address: 8 Drumahiskey Road

Balnamore Ballymoney County Antrim BT53 7QL

Contact(s): Carin Cornwall

Colm Hurley
Darren O'Mahony
Gabriella Horan
Joe Gervin
John Cameron
Lucy Newland
Martin Gardiner
Matthew Gilbert
Neil Haggan
Paul Dunlop
Sean Ross
Stephen Franey

Stephen McCracken Stephen Watson Stuart Abraham Thomas McAllis

**Project** 20-0399B Bus Connects Route 6

Quotation No.: Q20-21063 Date Received: 29-Oct-2020

Order No.: Date Instructed: 29-Oct-2020

No. of Samples: 1

Turnaround (Wkdays): 5 Results Due: 04-Nov-2020

Date Approved: 04-Nov-2020

Approved By:

**Details:** Glynn Harvey, Technical Manager



Eurofins Chemtest Ltd Depot Road Newmarket CB8 0AL

Tel: 01638 606070

Email: info@chemtest.com

Client: Causeway Geotech Ltd		20-29273				
Quotation No.: Q20-21063	(		st Sam		1088370	
		Sa	ocation:	R6-CP10		
		Sample Type				
		Top Depth (m):				
			Date Sa	ampled:	24-Oct-2020	
			Asbest	os Lab:	COVENTRY	
Determinand	Accred.	SOP	Units	LOD		
ACM Type	U	2192		N/A	-	
Asbestos Identification	U	2192		N/A	No Asbestos Detected	
ACM Detection Stage	U	2192		N/A	-	
Moisture	N	2030	%	0.020	11	
рН	U	2010		4.0	8.3	
Boron (Hot Water Soluble)	U	2120	mg/kg	0.40	< 0.40	
Sulphate (2:1 Water Soluble) as SO4	U	2120	g/l	0.010	0.50	
Cyanide (Total)	U	2300	) י	0.50	< 0.50	
Arsenic	U	2450	mg/kg	1.0	18	
Cadmium	U	2450	mg/kg	0.10	1.6	
Chromium	U	2450	mg/kg	1.0	16	
Copper	U	2450	mg/kg	0.50	24	
Mercury	U	2450	mg/kg	0.10	< 0.10	
Nickel	U	2450	mg/kg	0.50	41	
Lead	U	2450	mg/kg	0.50	27	
Zinc	U	2450	mg/kg	0.50	79	
Organic Matter	U	2625	%	0.40	1.9	
Total TPH >C6-C40	U	2670	0 0	10	230	
Naphthalene	U	2700	0 0	0.10	9.2	
Acenaphthylene	U	2700	0	0.10	0.57	
Acenaphthene	U		mg/kg		5.7	
Fluorene	U	2700	mg/kg		4.8	
Phenanthrene	U	2700		0.10	18	
Anthracene	U	2700	mg/kg	0.10	3.1	
Fluoranthene	U	2700	mg/kg	0.10	11	
Pyrene	U		mg/kg		10	
Benzo[a]anthracene	U	2700	mg/kg		4.5	
Chrysene	U	2700	0	0.10	4.7	
Benzo[b]fluoranthene	U	2700	0	0.10	4.8	
Benzo[k]fluoranthene	U	2700	5	0.10	2.0	
Benzo[a]pyrene	U		mg/kg		4.0	
Indeno(1,2,3-c,d)Pyrene	U		mg/kg		2.1	
Dibenz(a,h)Anthracene	U		mg/kg		0.98	
Benzo[g,h,i]perylene	U	2700	0 0	0.10	2.2	
Coronene	N	2700	0	0.10	< 0.10	
Total Of 17 PAH's	N		mg/kg		88	
Total Phenols	U	2920	mg/kg	0.30	< 0.30	

# **Test Methods**

SOP	Title	Parameters included	Method summary
2010	pH Value of Soils	рН	pH Meter
2030	Moisture and Stone Content of Soils(Requirement of MCERTS)	Moisture content	Determination of moisture content of soil as a percentage of its as received mass obtained at <37°C.
2040	Soil Description(Requirement of MCERTS)	Soil description	As received soil is described based upon BS5930
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2700	Speciated Polynuclear Aromatic Hydrocarbons (PAH) in Soil by GC-FID	Acenaphthene; Acenaphthylene; Anthracene; Benzo[a]Anthracene; Benzo[a]Pyrene; Benzo[b]Fluoranthene; Benzo[ghi]Perylene; Benzo[k]Fluoranthene; Chrysene; Dibenz[ah]Anthracene; Fluoranthene; Fluorene; Indeno[123cd]Pyrene; Naphthalene; Phenanthrene; Pyrene	Dichloromethane extraction / GC-FID (GC-FID detection is non-selective and can be subject to interference from co-eluting compounds)
2920	Phenols in Soils by HPLC	Phenolic compounds including Resorcinol, Phenol, Methylphenols, Dimethylphenols, 1- Naphthol and TrimethylphenolsNote: chlorophenols are excluded.	60:40 methanol/water mixture extraction, followed by HPLC determination using electrochemical detection.

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# APPENDIX H SPT HAMMER ENERGY MEASUREMENT REPORT





# **SPT Hammer Energy Test Report**

in accordance with BSEN ISO 22476-3:2005

Southern Testing Keeble House Stuart Way East Grinstead West Sussex SPT Hammer Ref: .0643

Test Date:

22/02/2020

Report Date:

03/03/2020

File Name:

.0643.spt

Test Operator:

NPB

#### **Instrumented Rod Data**

Diameter d<sub>r</sub> (mm):

**RH19 4QA** 

54

Wall Thickness  $t_r$  (mm):

6.0

Assumed Modulus Ea (GPa): 200

Falling Height h (mm): 760

SPT String Length L (m): 10.0

**SPT Hammer Information** 

Hammer Mass m (kg): 63.5

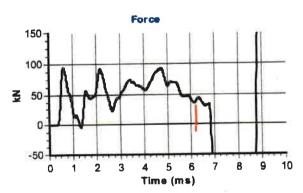
Accelerometer No.1: Accelerometer No.2:

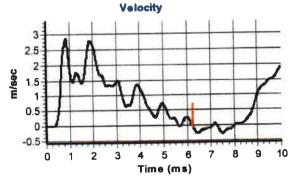
6458

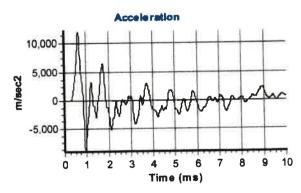
9607

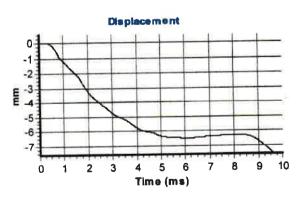
**Comments / Location** 

**BALLEYMONEY** 









#### **Calculations**

Area of Rod A (mm2):

905

Theoretical Energy  $E_{theor}$  (J):

473

Measured Energy  $E_{meas}$  (J):

400

Energy Ratio E, (%):

85

Signed: Neil Burrows

Title:

Field Operations Manager

The recommended calibration interval is 12 months



# **SPT Hammer Energy Test Report**

in accordance with BSEN ISO 22476-3:2005

**Southern Testing Keeble House Stuart Way East Grinstead** West Sussex

**RH19 4QA** 

SPT Hammer Ref: .17

Test Date:

22/02/2020

Report Date:

03/03/2020

File Name:

.T7.spt

Test Operator:

**NPB** 

### **Instrumented Rod Data**

Diameter d<sub>r</sub> (mm):

54

Wall Thickness t<sub>r</sub> (mm):

6.0

Assumed Modulus Ea (GPa): 200

Accelerometer No.1:

6458

Accelerometer No.2:

9607

### **SPT Hammer Information**

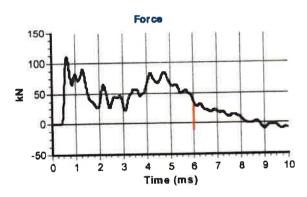
Hammer Mass m (kg):

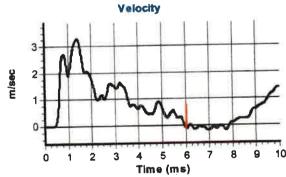
Falling Height h (mm): 760

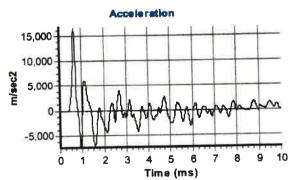
SPT String Length L (m): 10.0

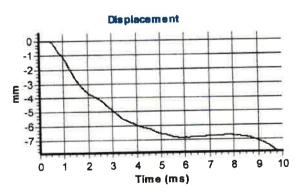
#### **Comments / Location**

**BALLEYMONEY** 









#### **Calculations**

Area of Rod A (mm2):

905

Theoretical Energy Etheor (J):

473

Measured Energy E<sub>meas</sub> (J):

399

Energy Ratio  $E_r$  (%):

84

Signed: **Neil Burrows** 

Title:

Field Operations Manager

The recommended calibration interval is 12 months